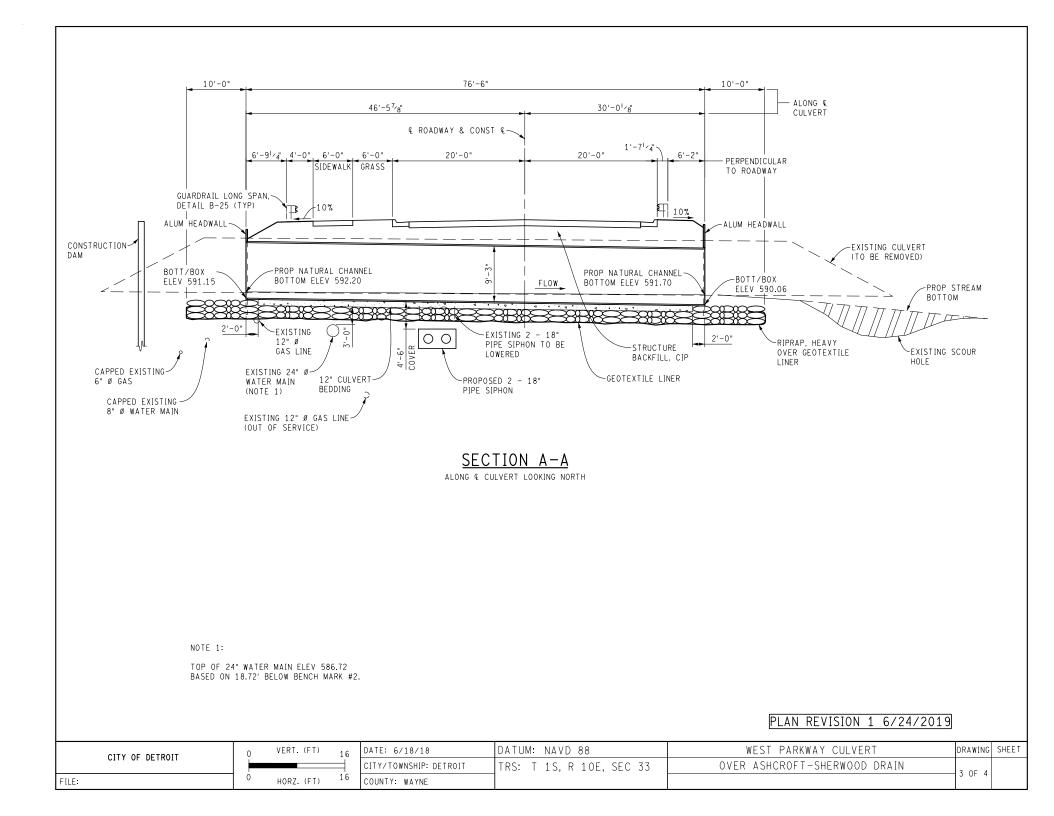
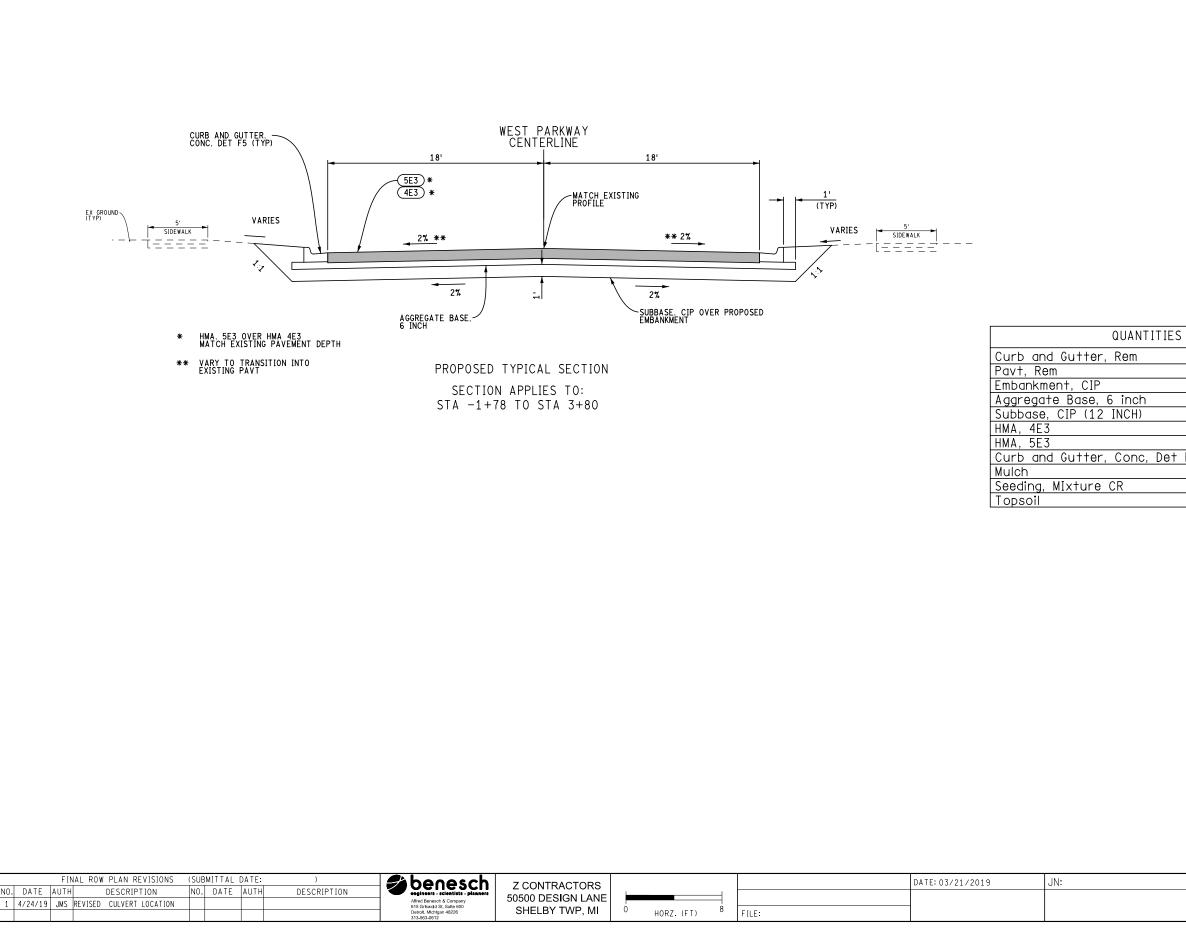


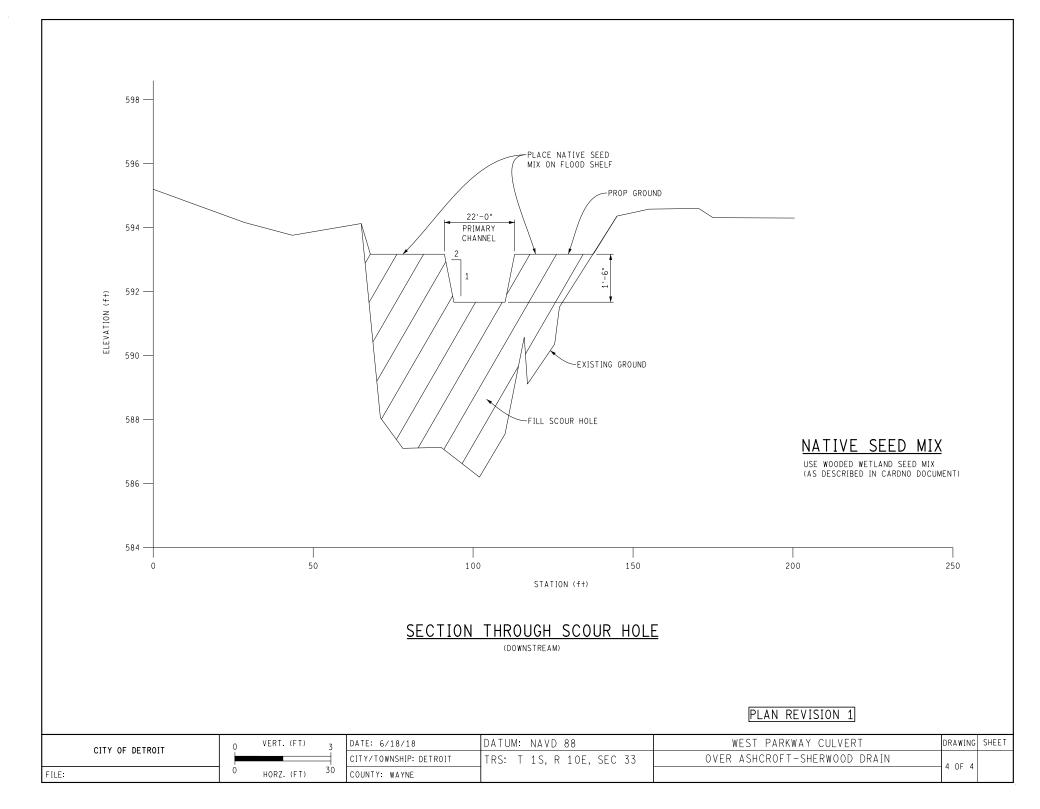
ALUM WINGWAL (TYP)	PROPOSED 22'-1" x 9'-3" ALUM BOX CULVERT EXISTING CULVERT (TO BE REMOVED) EXIST SIPHON (TO BE REMOVED AND RELOCATED)	25'-6" ALUM HEADWALL 22'-1" CLR SPAN TRANSVERSE &		ELEV 599.31 EAST END 600.40 WEST END ON 3.5 RIPRAP, HEAVY OVER GEOTEXTILE LINER	GUARDRAIL LONG SPAN, DETAIL B-25	
CITY OF DETROIT	PLAN REVISION 1 VERT. (FT) 12 DATE: 6/18/18	6/24/2019 ARE REGU	Ve ELEV. AT U/S D/S E FACE OF CHANNEL STRUCTURE (FPS) S 605.92 9.03 605.45 9.07 S water Surface AND/OR ENE S S S 10 ATER SURFACE AND/OR ENE F I S S	SLOPE RIPRAP AT 1 TOWARDS UTILITY P 	VE: ALUMINUM BOX WATER VELOCITY CHANGE IN SURFACE IN WS U/S OF ELEV. AT U/S D/S EXISTING FACE OF CHANNEL STRUCTURE STRUCTURE (FPS) & VE 605.31 7.24 -0.61 606.01 7.1 -0.44 HE ABOVE HYDRAULIC TABLE BE USED FOR ESTABLISHING A HEY ARE VERIFIED WITH THE WVIRONMENTAL OUALITY.	CHANGE IN WS U/S OF PROPOSED STRUCTURE & VE -0.39 -0.45
	12	TRS: T 1S, R 10E, SEC		OVER ASHCROFT-SHE		DRAWING SHE

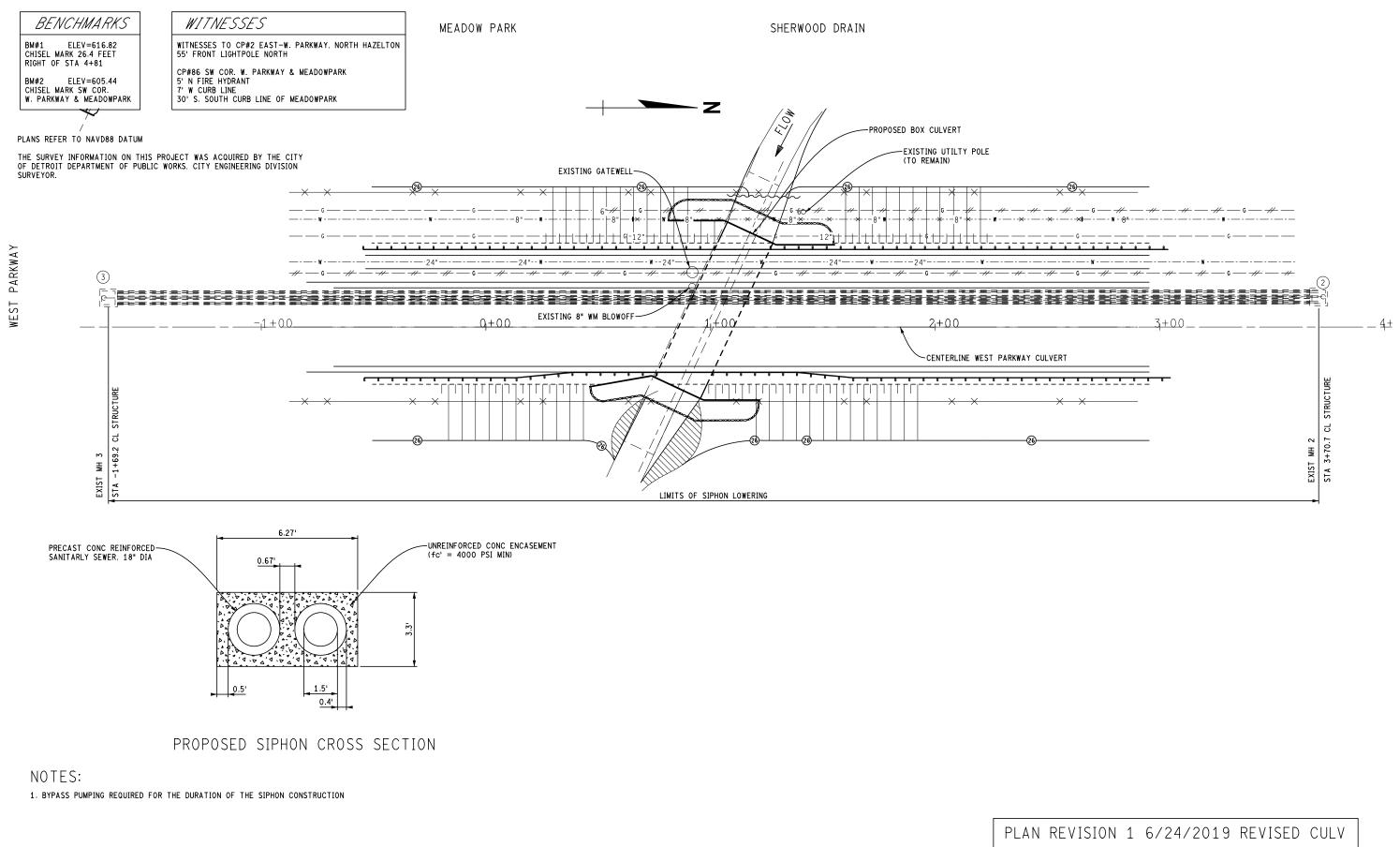




ES		
	687	FT
	1374	SYD
	569	CYD
	1603	SYD
	560	CYD
	280	TON
	290	TON
et F5	688	FT
	229	SYD
	70	LB
	40	CYD

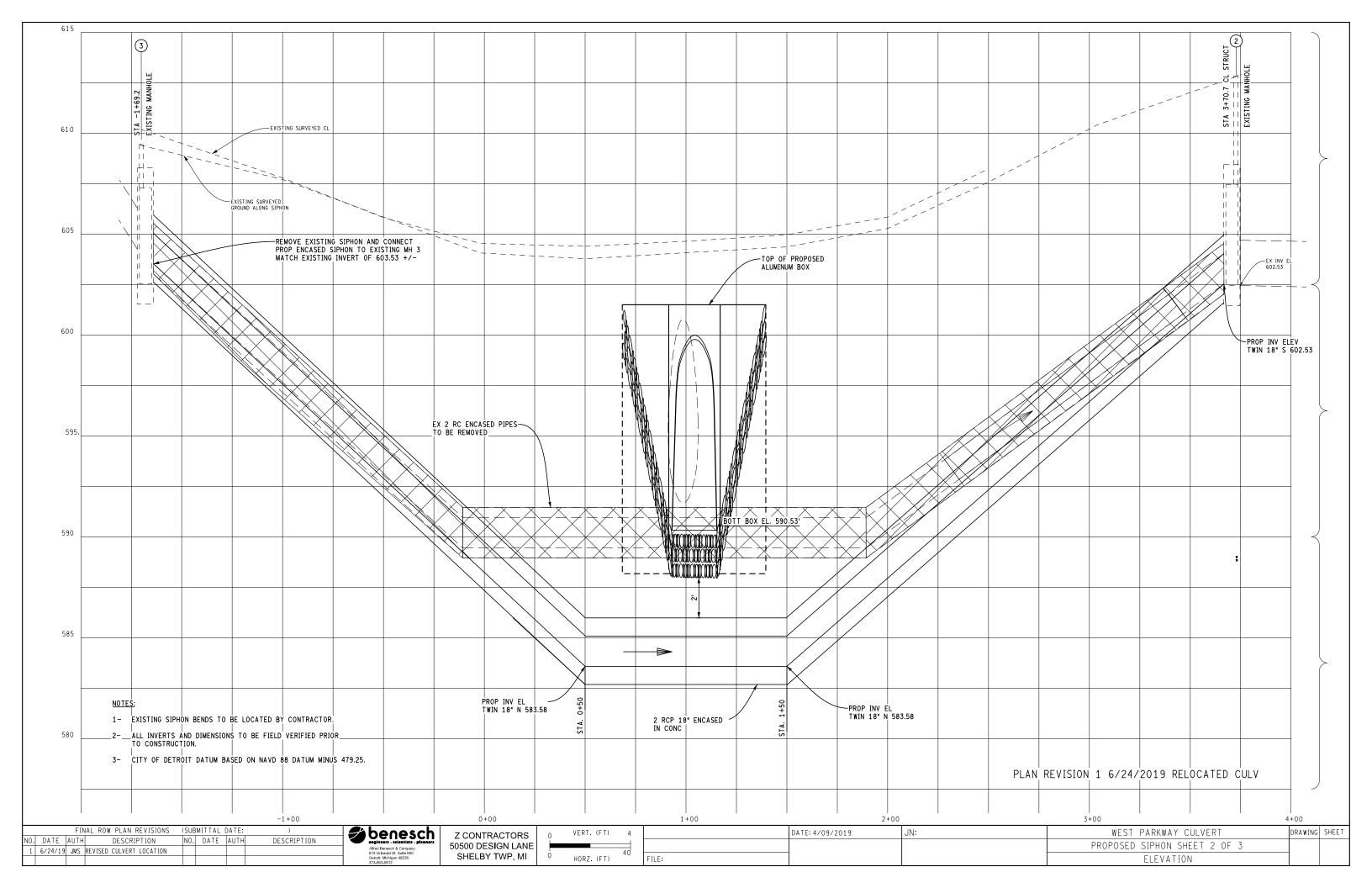
WEST PARKWAY CULVERT	DRAWING	SHEET
PROPOSED SIPHON SHEET 3 OF 3		
ROAD TYPICAL AND QUANTITIES		

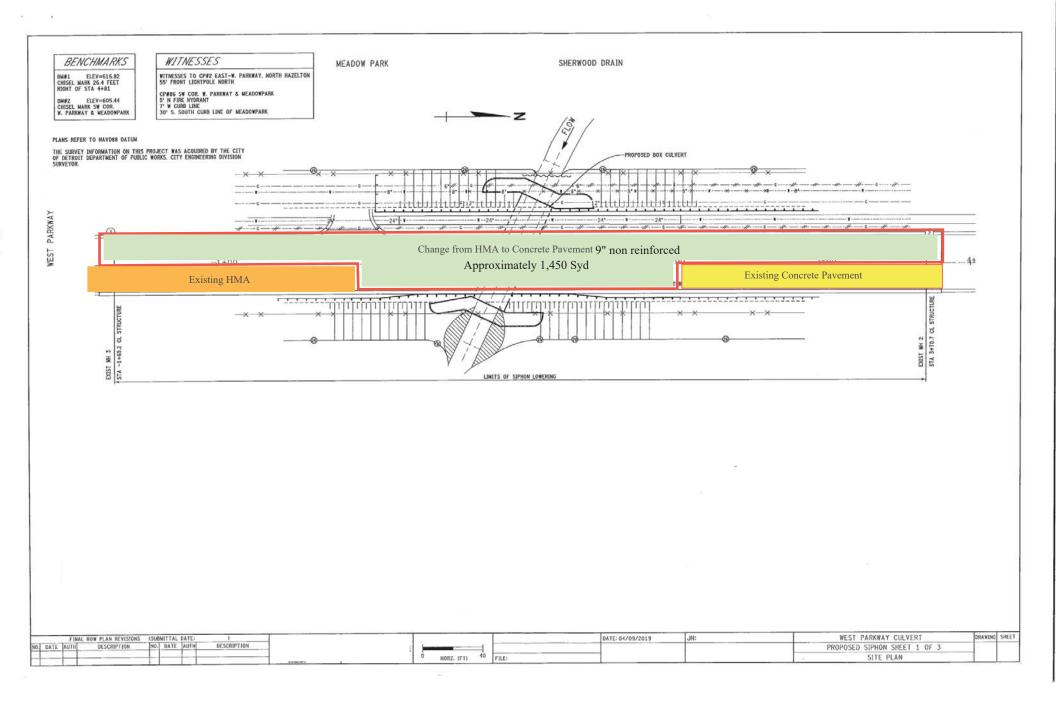


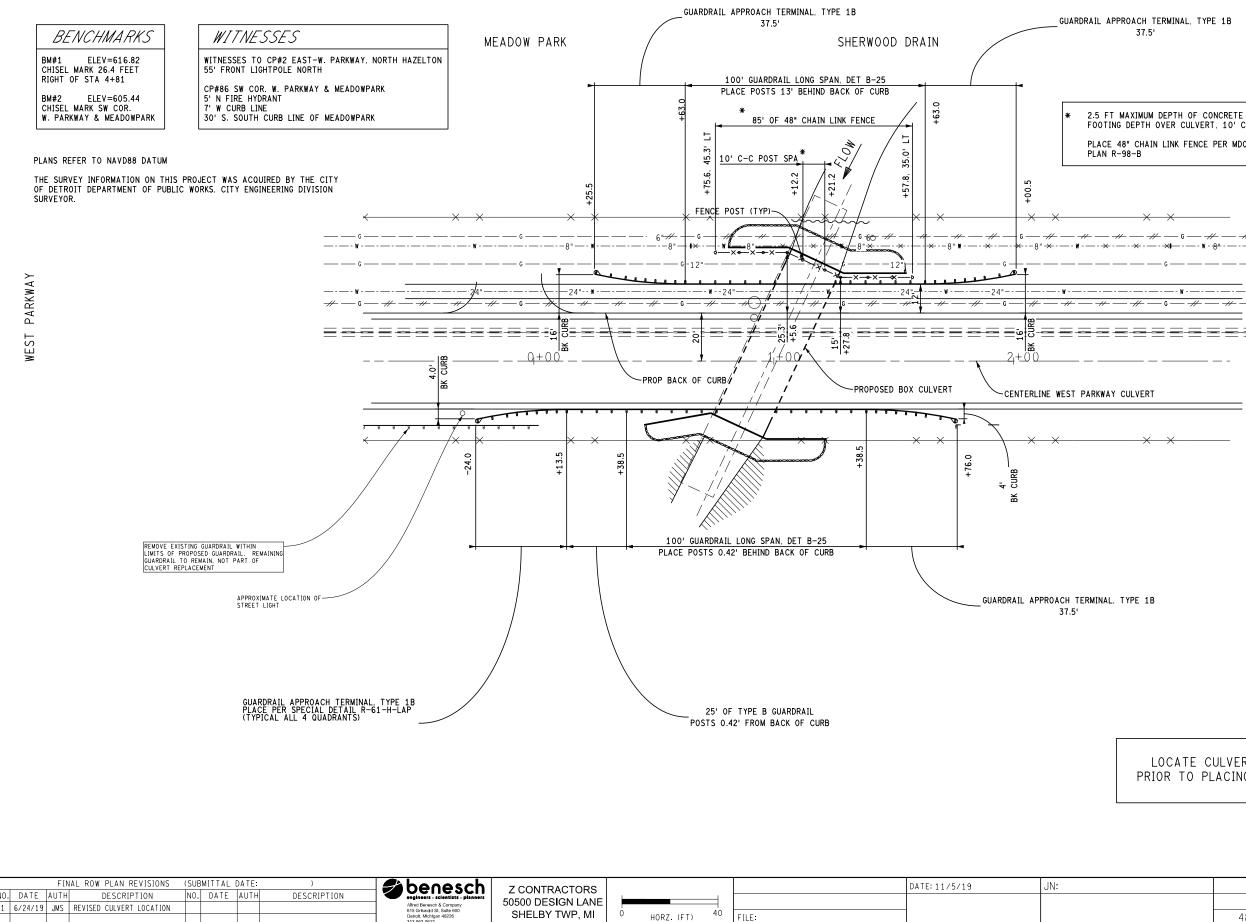


	FI	NAL ROW PLAN REVISIONS	(SUBMITTAL	DATE)	💋 benesch	Z CONTRACTORS					DATE: 04/09/2019	JN:
N0.	DATE AUTH	DESCRIPTION	NO. DATE	AUTH	DESCRIPTION	engineers - scientists - planners	50500 DESIGN LANE						
1	6/24/19 JMS	REVISED CULVERT LOCATION				Alfred Benesch & Company 615 Griswold St, Suite 600	SHELBY TWP, MI	6		40		-	
						Detrolt, Michigan 48226 313-963-0612	SHEEDT TWP, MI	Ū	HORZ.(FT)	10	FILE:		

WEST PARKWAY CULVERT	DRAWING	SHEET
PROPOSED SIPHON SHEET 1 OF 3		
SITE PLAN		







PTH OF CONCRETE R CULVERT, 10' C-C SPA	
NK FENCE PER MDOT STD	

— - — - ¥ - — - Z

LOCATE CULVERT TIE BACK STRAPS / RODS PRIOR TO PLACING GUARDRAIL AND FENCE POSTS

WEST PARKWAY CULVERT	DRAWING	SHEET
GUARDRAIL LAYOUT SHEET		
48" CHAIN LINK FENCE LAYOUT (WEST SIDE)		
-		

MPORTANT:

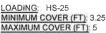
ASSEMBLY INSTRUCTIONS WILL BE SHIPPED WITH THE STRUCTURE. THEY ARE LOCATED IN THE BRIGHTLY COLORED BOLT KEG.

NOTES: 1. INVERT END OF HAUNCH PLATES PAINTED RED. WHEN A FULL CORRUGATED INVERT IS USED, IT IS ANTICIPATED THAT NO BACKFILL MATERIAL WILL BE PLACED INSIDE THE STRUCTURE AND ON TOP OF THE INVERT PLATES. THEREFORE, SCALLOPED CLOSURE PLATES ARE TO BE CLIPPED TO THE OUTSIDE OF THE RECEIVING CHANNEL TO MINIMIZE BACKFILL INFILTRATION IN THE VALLEYS OF THE CORRUGATED INVERT PLATE BELOW THE RECEIVING CHANNEL. NOTE: THIS DOES NOT MAKE A JOINT TIGHT ENOUGH TO PREVENT INFILTRATION OF FINE SILTS OR SANDS. THE USE OF A GEOTEXTILE PREVENTS THE INFILTRATION OF THE BACKFILL THROUGH THE UNFILLED BOLT HOLES AND THE SPACE BETWEEN THE INVERT PLATE AND THE SCALLOP PLATE, A ROLL OF GEOTEXTILE IS PROVIDED FOR THIS PURPOSE, WHEN SHORT FOOTING PADS ARE USED, THE SCALLOPED CLOSURE PLATES ARE NOT PROVIDED UNLESS ORDERED AS AN EXTRA, SINCE IT IS ANTICIPATED THESE FOOTING PLATES WILL BE BURIED.

INVERT END OF HAUNCH RIBS PAINTED RED.

GENERAL NOTES

CONFIRMATION OF COVER - THIS STRUCTURE IS WITHIN THE MINIMUM AND MAXIMUM ALLOWABLE HEIGHT OF COVER. FOR THE DESIGNATED LOADING, AS FOLLOWS:



2. FOR PROPER BOLT SIZE USAGE, REFER TO THE FOLLOWING:

PLATE ONLY

	1 PLATE	2 PLATE	3 PLATE	4 PLATE
0 100" - 0 125" THK PLATE		1 1/4"	1 1/4"	1 1/2"
0.150" - 0.175" THK PLATE		1 1/4"	1 1/2"	2"
0.200" - 0.250" THK, PLATE		1 1/2"	2*	2"

	1 PLATE	2 PLATE	3 PLATE	4 PLATE
0.100" - 0.125" THK, PLATE	1 1/4"	1 1/2"	1 1/2"	2*
0 150" - 0 175" THK PLATE	1 1/4"	1-1/2"	2*	2*
0 200" - 0.225" THK, PLATE	1 1/2"	2"	2*	2"
0.250" THK. PLATE	1 1/2"	2*	2*	2 1/2"

PLATE W/ T6 REINFORCING RIB

ce to the project owner, engineer and contractor to riginoened Solutions LLC ("Contech") Neither th for any part thereof, may be used, reproduced or

work progresses, these discrepancies must be repo

	1 PLATE	2 PLATE	3 PLATE	4 PLATE
0.100" - 0.125" THK. PLATE	1 1/2"	2*	2"	2*
0.150" - 0.175" THK PLATE	2*	2*	2"	2 1/2"
0.200" - 0.225" THK, PLATE	2*	2"	2 1/2"	2 1/2"
0.250" THK_PLATE	2"	2"	2 1/2"	2 1/2"

- NUTS MAY BE LOCATED ON STRUCTURE'S INTERIOR OR EXTERIOR TO ALLOW CONVENTIONAL ACCESS DURING ASSEMBLY AND TORQUING. ONLY ONE SIDE OF NUT HAS A CURVED SURFACE AND IT SHOULD BE IN DIRECT CONTACT WITH PLATE VALLEY.
- ALL PLATE LAPS AND REINFORCING RIBS MUST BE PROPERLY MATED IN A TANGENT 2 FASHION USING PROPER ALIGNMENT TECHNIQUES AND HELD IN ALIGNMENT BY FASTENERS (FINGER TIGHTENED ONLY) BEFORE BACKFILLING COMMENCES, ALL FASTENERS MUST BE TORQUED FOR ADEQUATE COMPONENT CONTACT. GOOD COMPONENT FIT IS BETTER THAN HIGH TOROUE.
- FASTENER TORQUE REQUIREMENTS: 0.100" THICK PLATE AT 90-155 FOOT-POUNDS. 3 FOR ALL THICKER PLATES AND REINFORCING RIBS, TORQUE AT 115-135 FOOT-POUNDS. TORQUE LEVELS ARE FOR INSTALLATION, NOT RESIDUAL, IN-SERVICE REQUIREMENTS. SINCE TOROUTING MAY LOOSEN PREVIOUSLY TIGHTENED FASTENERS, MULTIPLE PASSES MAY BE NECESSARY, WHEN SEAM SEALANT TAPE IS USED, FASTENERS SHOULD BE TORQUED AGAIN AFTER 24 HOURS
- ALL ALUMINUM STRUCTURAL PLATE MATERIAL IS MANUFACTURED IN ACCORDANCE WITH AASHTO M219, ASTM B746 AND ASTM B864 SPECIFICATIONS. SEE ASSEMBLY INSTRUCTIONS SHIPPED WITH MATERIAL IN FASTENER CONTAINER, ALSO REFER TO SPECIFIC PRODUCT CATALOG FOR ADDITIONAL PRODUCT INFORMATION.

THE ASSEMBLY BOLTS AND NUTS ARE SPECIALLY DESIGNED WITH ROUNDED OR SPHERICAL THROATS FOR FITTING EITHER THE CREST OR VALLEY OF THE CORRUGATIONS, PROVIDING MAXIMUM BEARING CONTACT AREA WITH THE PLATES WITHOUT THE USE OF WASHERS. NOTE THAT THE BOLTS AND NUTS SHOULD BE INSTALLED SUCH THAT THE ROUNDED PORTION IS IN CONTACT WITH THE PLATES.

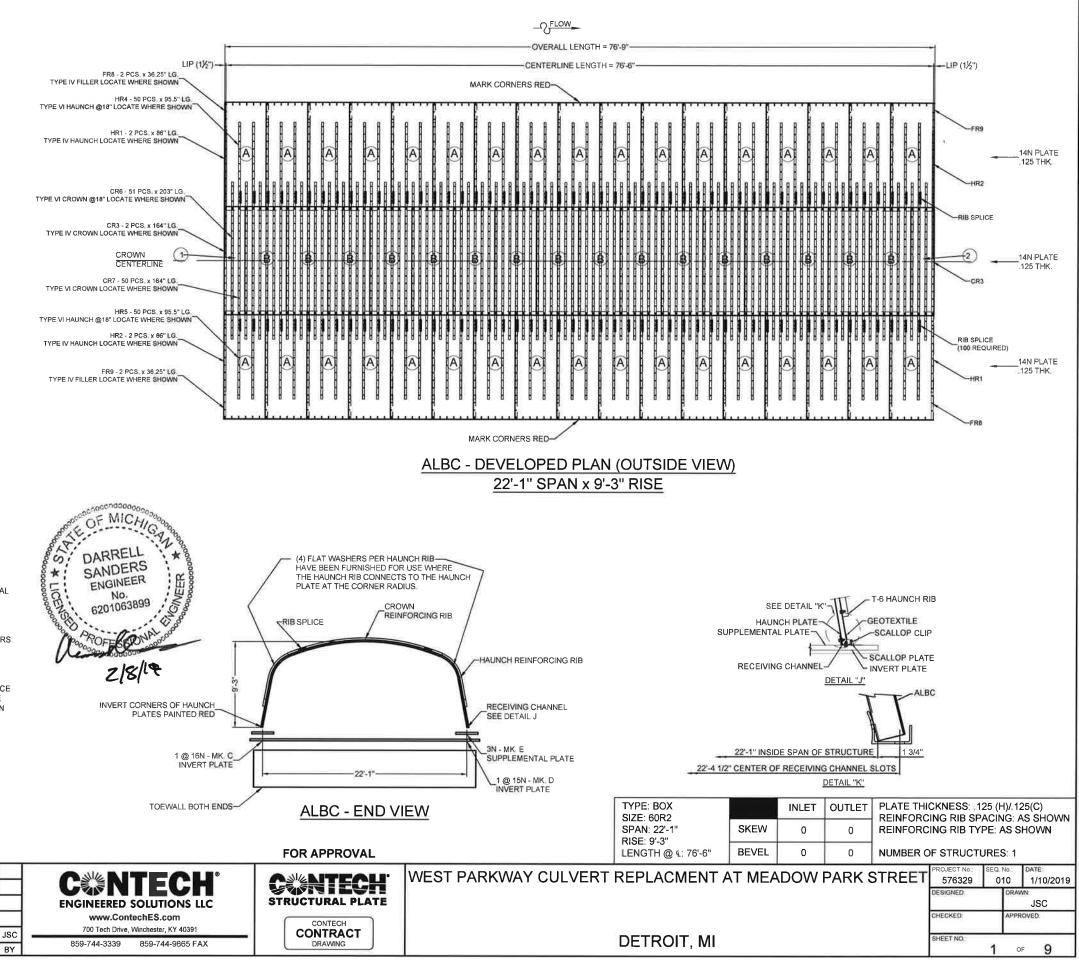
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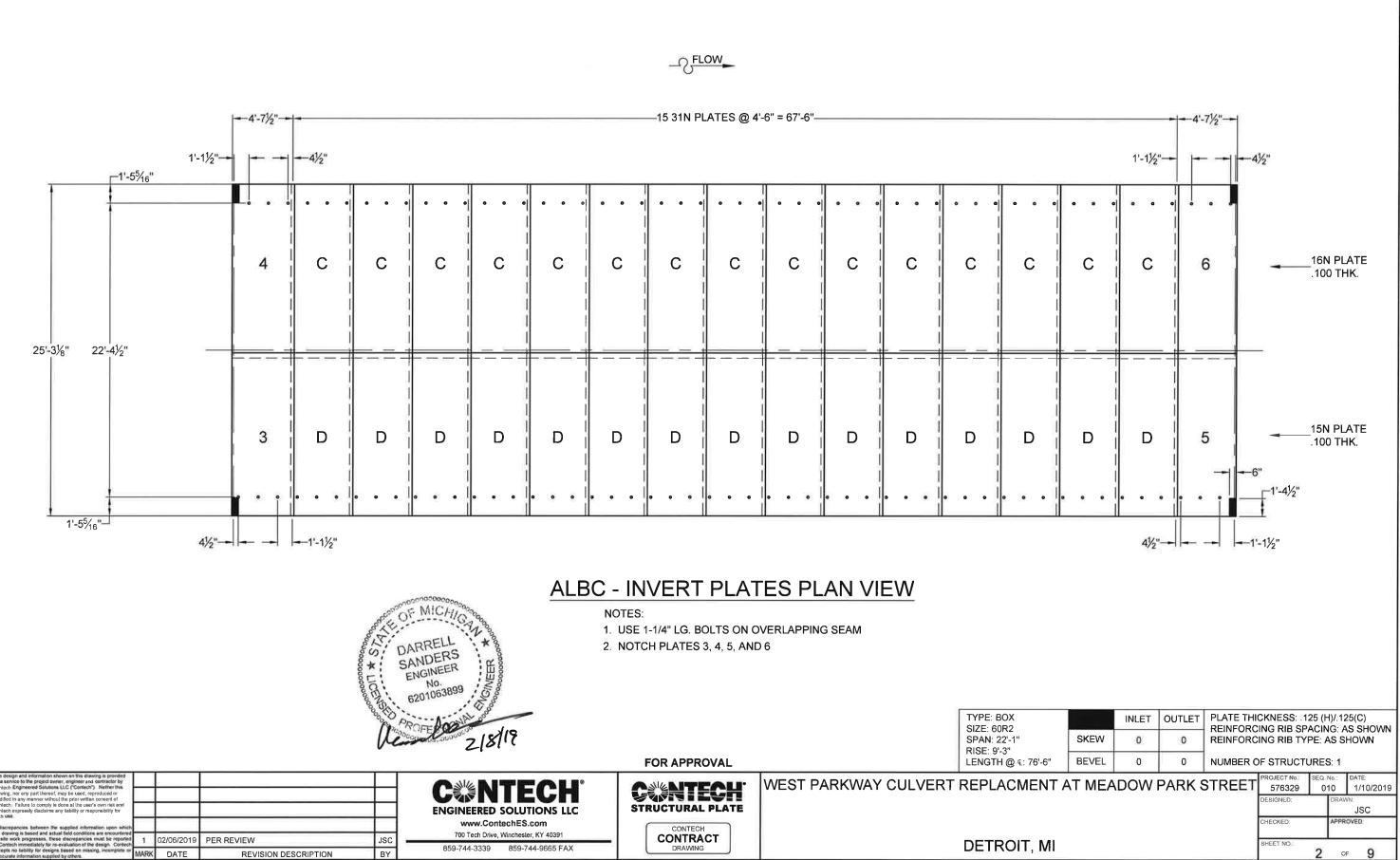
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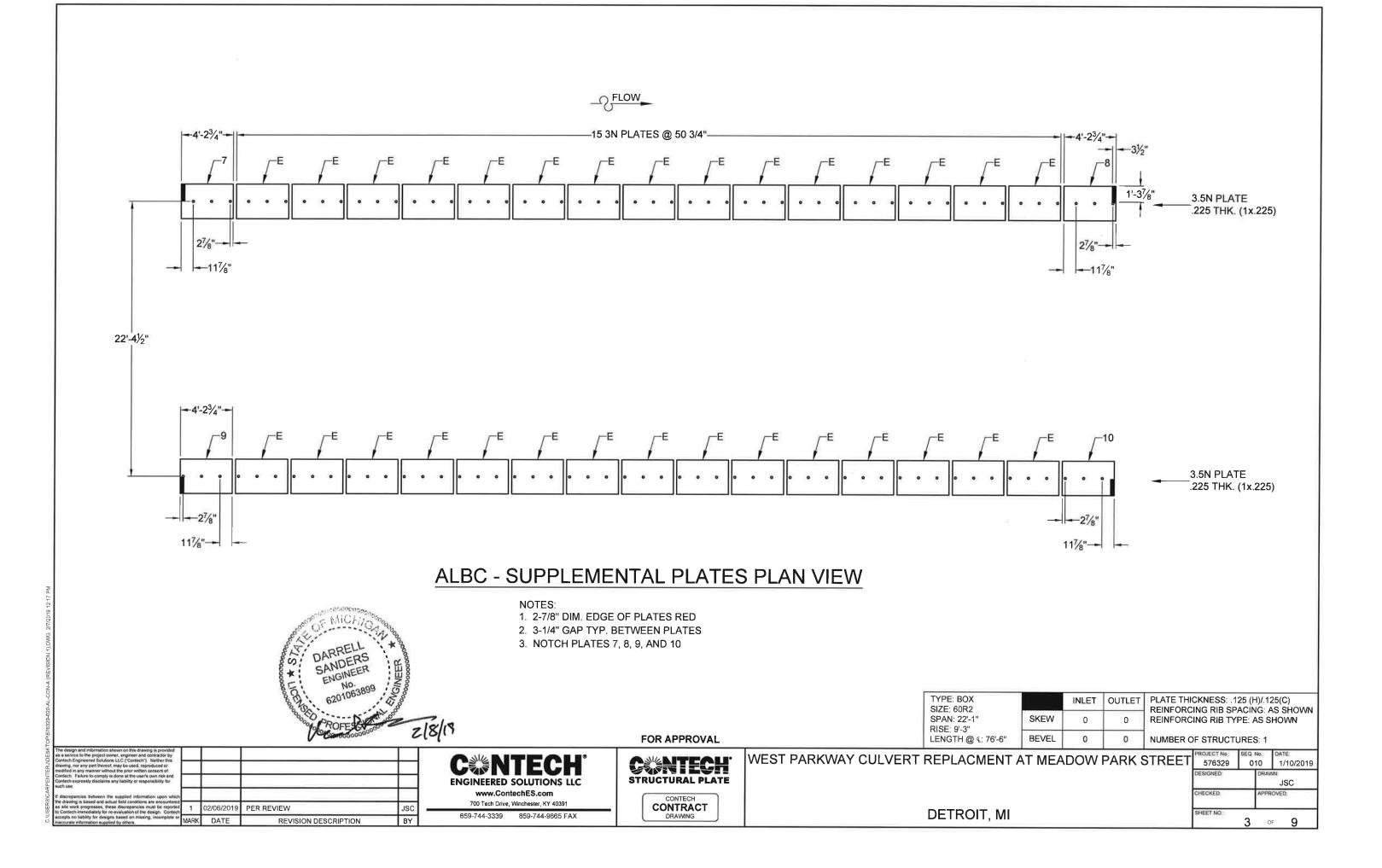
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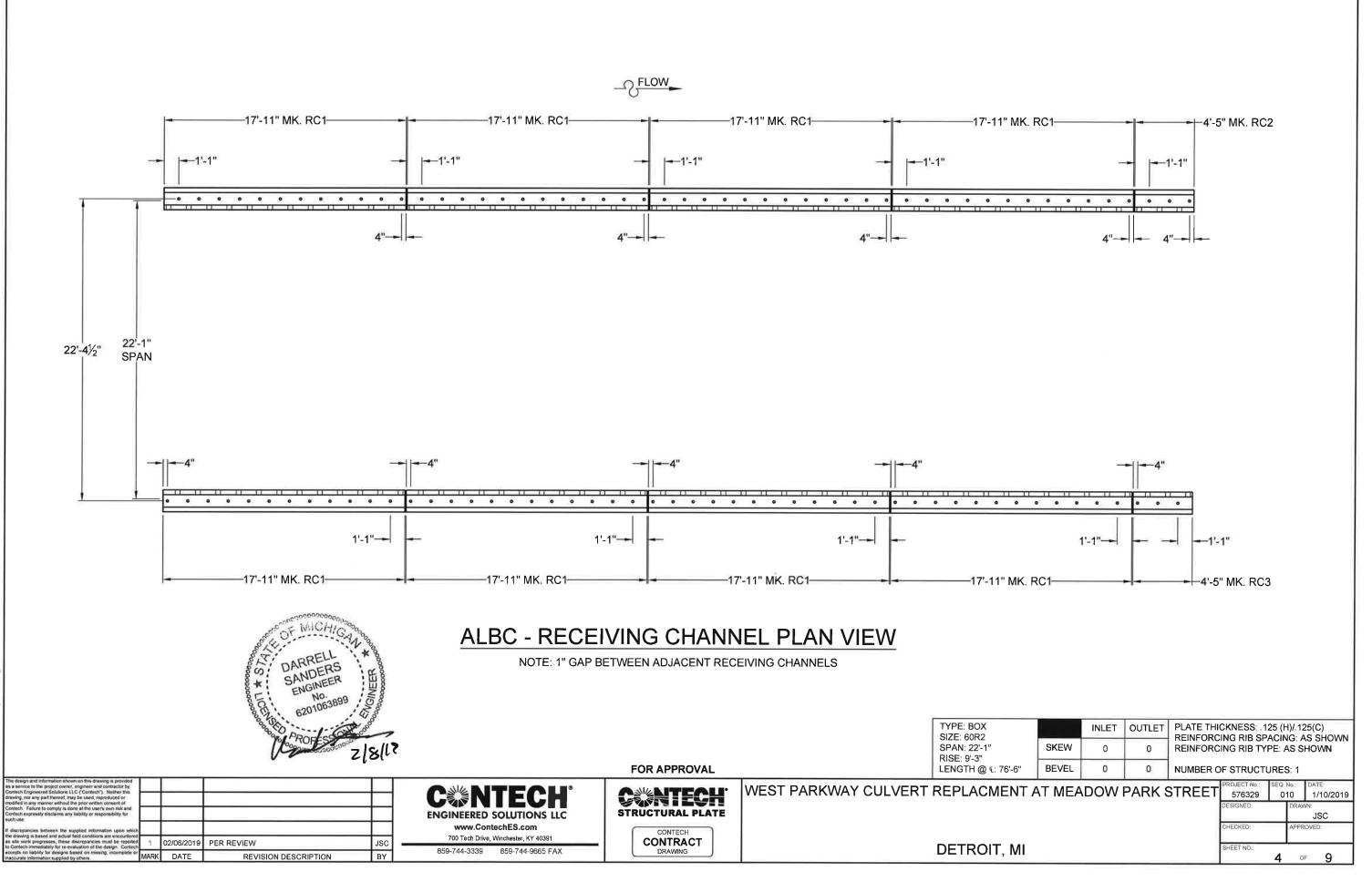
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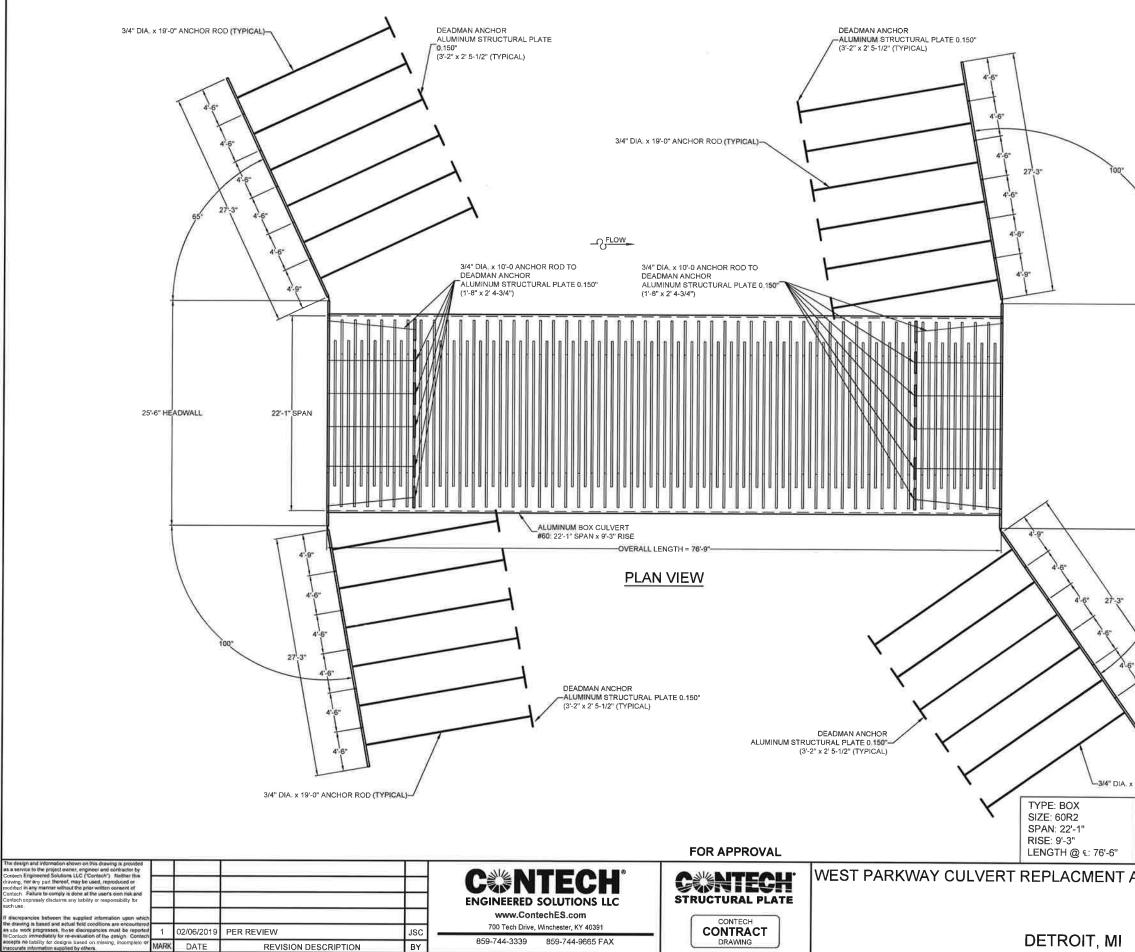


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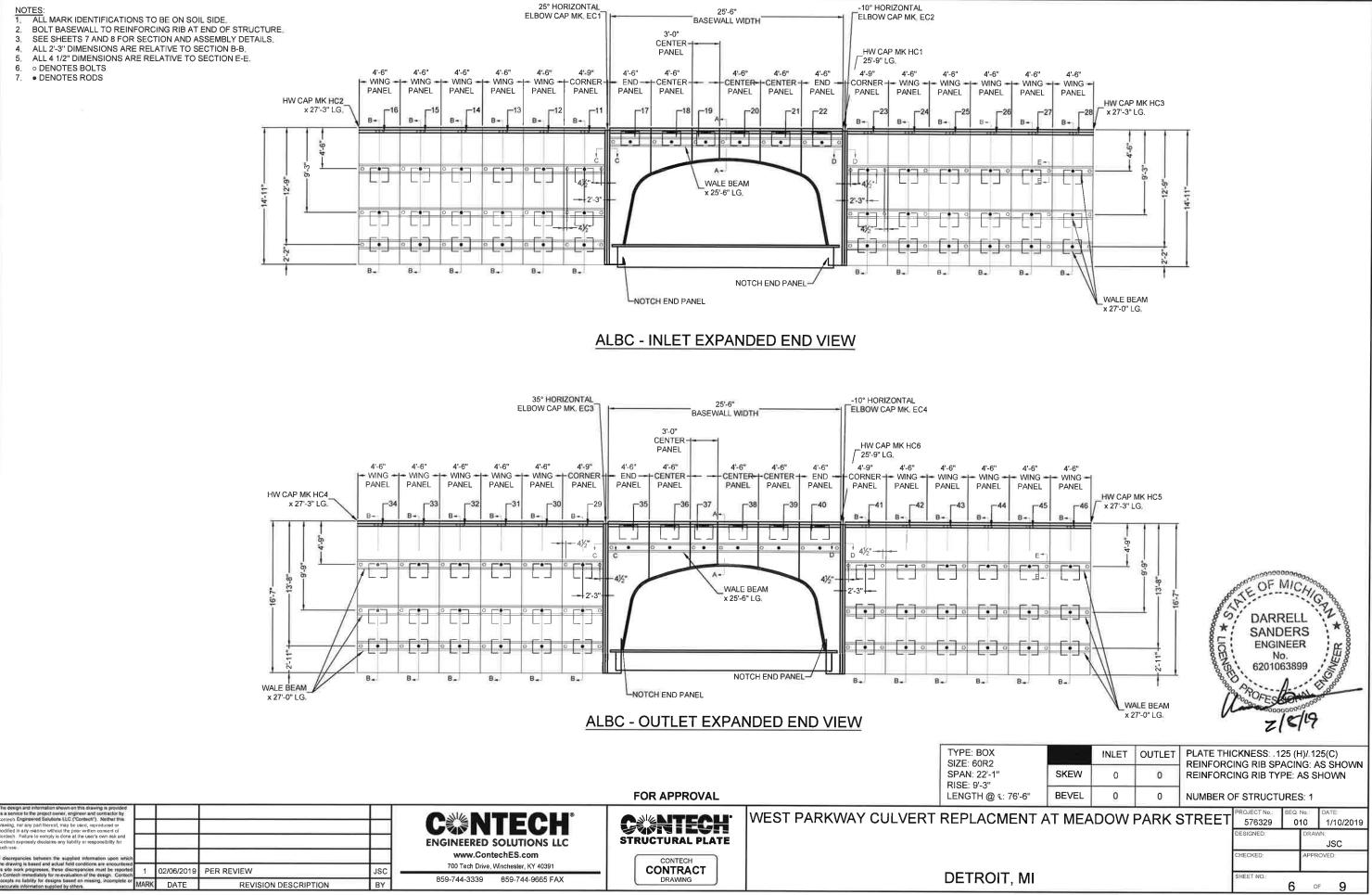


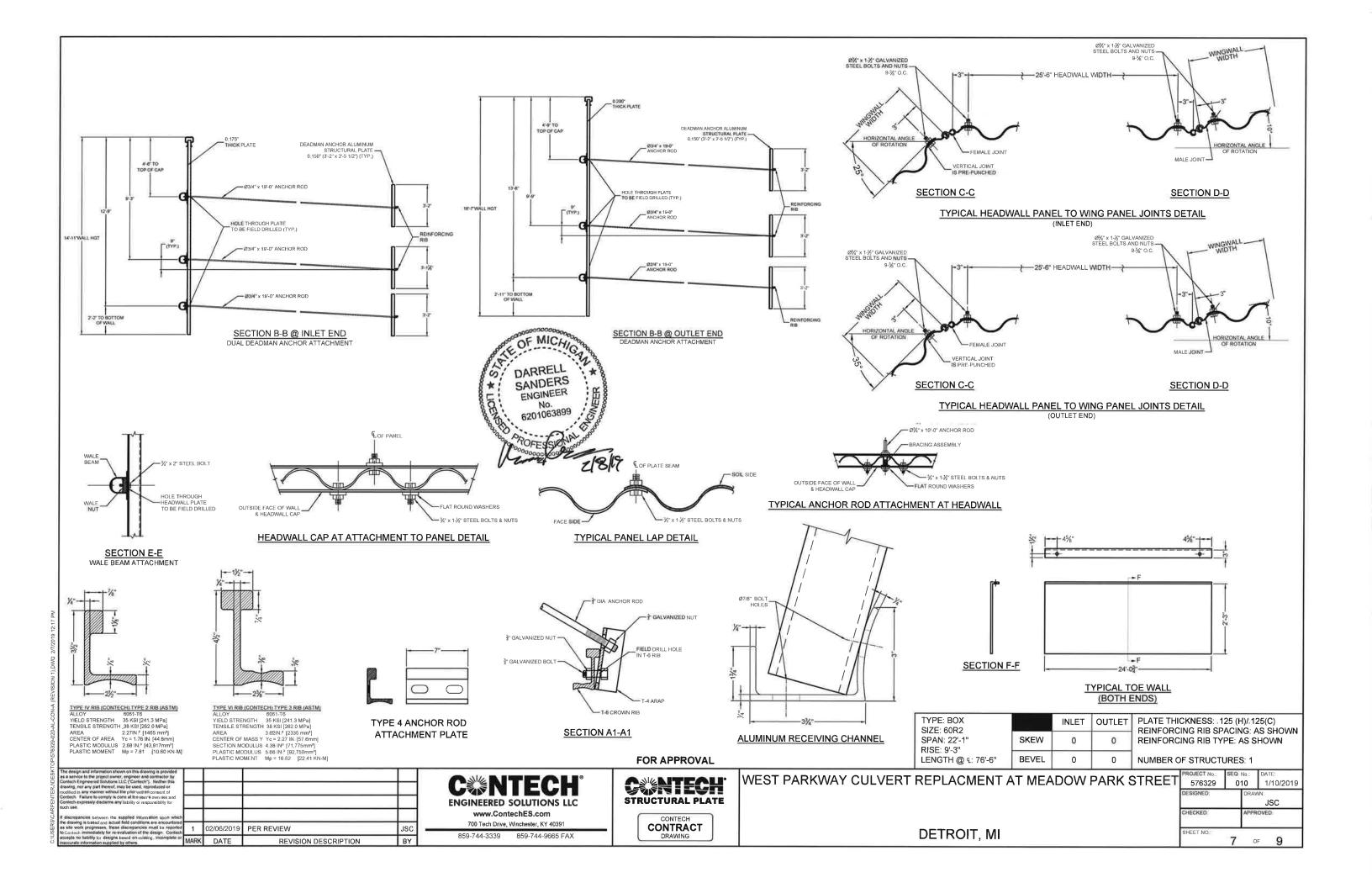


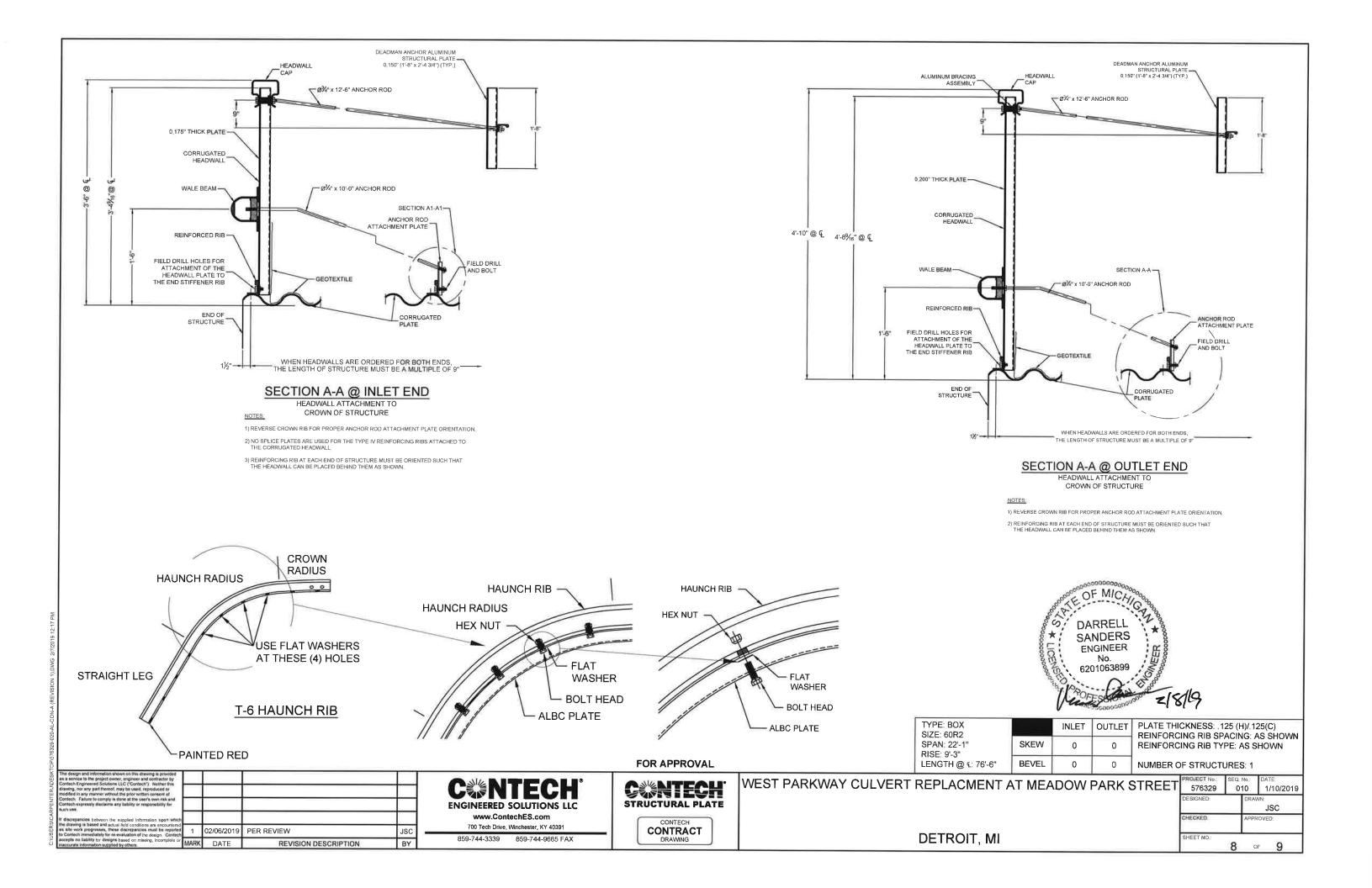


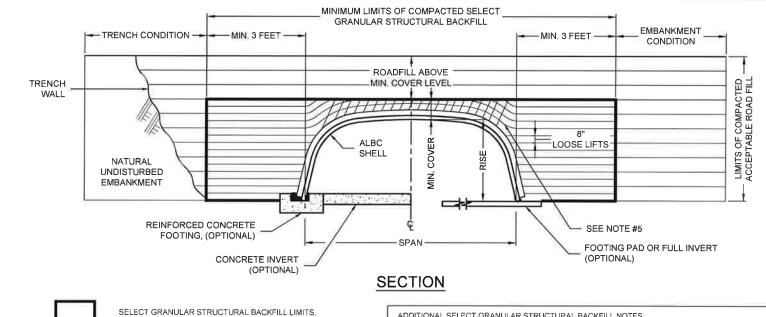


25'-6" HEAI	DWALL		
55*			opport OF MICHICOgg
x 19-0" ANCHOF	ROD (TYPIC)		DARRELL SANDERS ENGINEER No. 6201063899
SKEW	INLET 0	OUTLET 0	PLATE THICKNESS: .125 (H)/.125(C) REINFORCING RIB SPACING: AS SHOWN REINFORCING RIB TYPE: AS SHOWN
BEVEL	0	0	NUMBER OF STRUCTURES: 1
AT MEA	DOW I	PARK	PROJECT No. SEQ. No. DATE: 576329 010 1/10/2019 DESIGNED: DRAWN JSC CHECKED: APPROVED: SHEET NO: 5 OF 9









ADDITIONAL SELECT GRANULAR STRUCTURAL BACKFILL NOTES:

SATISFACTORY BACKFILL MATERIAL, PROPER PLACEMENT, AND COMPACTION ARE KEY FACTORS IN OBTAINING MAXIMUM STRENGTH AND STABILITY

THE BACKFILL MATERIAL SHOULD BE FREE OF ROCKS, FROZEN LUMPS, AND FOREIGN MATERIAL THAT COULD CAUSE HARD SPOTS OR DECOMPOSE TO CREATE VOIDS. BACKFILL MATERIAL SHOULD BE WELL GRADED GRANULAR MATERIAL THAT MEETS THE REQUIREMENTS OF AASHTO M-145 FOR SOIL CLASSIFICATIONS A-1, A-2-4, A-2-5, OR A-3 MODIFIED,

SEE THE STRUCTURAL PLATE BACKFILL GROUP CLASSIFICATION TABLE ON THIS SHEET. BACKFILL MUST BE PLACED SYMMETRICALLY ON EACH SIDE OF THE STRUCTURE IN 8" LOOSE LIFTS: EACH LIFT IS TO BE COMPACTED TO A MINIMUM OF 90% DENSITY PER AASHTO T-180

A HIGH PERCENTAGE OF SILT OR FINE SAND IN THE NATIVE SOILS SUGGESTS THE NEED FOR A WELL GRADED GRANULAR BACKFILL MATERIAL TO PREVENT SOIL MIGRATION. IF THE PROPOSED BACKFILL IS NOT A WELL-GRADED MATERIAL, A NON-WOVEN GEOTEXTILE FILTER FABRIC SHALL BE PLACED BETWEEN THE SELECT BACKFILL AND THE IN SITU MATERIAL

DURING BACKFILL, ONLY LIGHTWEIGHT TRACKED VEHICLES (D-4 OR LIGHTER) SHOULD BE NEAR THE STRUCTURE AS FILL PROGRESSES ABOVE THE CROWN AND TO THE FINISHED GRADE. THE ENGINEER AND CONTRACTOR ARE CAUTIONED THAT THE MINIMUM COVER MAY NEED TO BE INCREASED TO HANDLE TEMPORARY CONSTRUCTION VEHICLE LOADS (HEAVIER THAN D-4).

Usual Materials	Stone Fragment, Gravel and Sand		Silty or Clayey Gravel and Sand		Coarse Sand
Plasticity Index	6 max.	6 max.	10 max.	10 max.	Non Plastic
Liquid Limits			40 max.	41 min.	
ļ.	Atterberg Limits fo	or Fraction Passin	g No. 40 (0.425 n	nm)	2
No. 200 (0.075 mm)	15 max.	25 max.	35 max.	35 max.	10 max.
No. 40 (0.425 mm)	30 max.	50 max.		*****	51 max.*
No. 10 (2.000 mm)	50 max.	2222			
Sieve Analysis Percent Passi	ng				
GROUP CLASSIFICATION	A-1-a	A-1-b	A-2-4	A-2-5	A-3
STRUCTURAL PLATE BACK	KFILL GROUP C	LASSIFICATION	REFERENCE A	ASHTO M-145	

*Modified from M-145

ice to the project owner, engineer and contractor b Engineered Solutions LLC ("Centech") Nether this

discrepancies botwaren the supplied movements re drawing in based and actual field conditions are encounter as site work progresses. These discrepancies must be respo-te contech immediately for re-invaluation of the design. Con the contech immediately for re-invaluation of the design. Con-text designs based on missing, incomplet

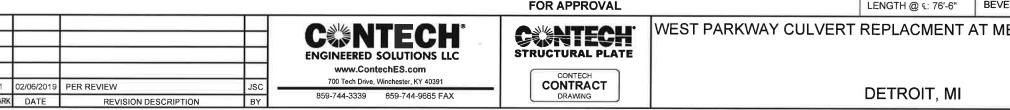
to comply is done at the user's own risk ar

Fine beach sands, windblown sands, stream deposited sands, etc., exhibiting fine, rounded particles and typically Classified by AASHTO M-145 as A-3 materials should not be used.

Reference the most current version of ASTM D2487, Standard Practice for Classification of Soils for Engineering Purposes (Unified Soil Classification System), for comparable soil groups,

OF MICHI DARRELL 0: SANDERS ENGINEER LICE NO. 6201063899 2/8/1

FOR APPROVAL



1.0 STANDARDS AND DEFINITIONS

- 1.1 STANDARDS All standards refer to the current ASTM/AASHTO edition Inless otherwise noted
- 1,1,1 ASTM B-864 "Standard Specification for Corrugated Aluminum Box Culve (AASHTO Designation M-219),
- 1,1.2 AASHTO Standard Specification for Highway Bridges Section 12 Division I Design, AASHTO LRFD Bridge Design Specifications Section 12.
- 1.1.3 AASHTO Standard Specification for Highway Bridges Section 26 Division II Construction, AASHTO LRFD Bridge Construction Specifications - Section 26, ASTM B789, Standard Practice for nstalling Corrugated Aluminum Structural Plate Pipe
- 1.2 DEFINITIONS
- 1.2.1 Owner In these specifications the word "Owner" shall mean Z Contractors Inc MI
- 1.2.2 Engineer In these specifications the word "Engineer" shall mean the Engineer of Record or Owner's designated engineering representativ
- 1.2.3 Manufacturer In these specifications the word "Manufacture shall mean CONTECH ENGINEERED SOLUTIONS 800-338-1122 Ryan Loeprich (616-403-5525)
- 1.2.4 Contractor In these specifications the word "Contractor" shall mean the firm or corporation undertaking the execution of any installation work under the terms of these specifications.
- 1.2.5 Approved In these specifications the word "approved" shall refer to the approval of the Engineer or his designated representative.
- As Directed In these specifications the words "as directed" shall refer to 1.2.6 the directions to the Contractor from the Owner or his designated representalive,
- 2.0 GENERAL CONDITIONS
 - 2.1 Any installation guidance provided herein shall be endorsed by the engineer; discrepancies herein are governed by the Engineer's plans and specifications
 - 2.2 The Contractor shall furnish all labor, material and equipment and perform all work and services except those set out and furnished by the Owner, necessary to complete in a satisfactory manner the site preparation, excavation, filling, compaction, grading as shown on the plans and as described therein. This work shall consist of all mobilization clearing and grading, grubbing, stripping, removal of existing material unless otherwise stated, preparation of the land to be filled, filling of the la spreading and compaction of the fill, and all subsidiary work necessary to complete the grading of the cut and fill areas to conform with the lines, grades, slopes, and specifications. This work is to be accomplished under the observation of the Owner or his designated representative.
 - 2.3 Prior to bidding the work, the Contractor shall examine, investigate and inspect the construction site as to the nature and location of the work, and the general and local conditions at the construction site, including without limitation, the character of surface or subsurface conditions and obstacles to be encountered on and around the construction site and shall make such additional investigation as he may deem necessary for the planning and proper execution of the work

If conditions other than those indicated are discovered by the Contractor, the Owner shall be notified immediately. The material which the Contractor believes to be a changed condition shall not be disturbed so that the owner can investigate the condition

> TYPE: BOX SIZE: 60R2 SPAN: 22'-1" RISE: 9'-3"

- 2.4 The construction shall be performed under the direction of the Engineer
- 2.5 All aspects of the structure design and site layout including foundations, backfill, end treatments and necessary scour consideration shall be performed by the Engineer.

NOTES:

- ALL SELECT GRANULAR BACKFILL TO BE PLACED IN A BALANCED 1. FASHION IN THIN LIFTS (8" LOOSE TYPICALLY) AND COMPACTED TO 90 PERCENT DENSITY PER AASHTO T-180.
- 2. COMPLETE AND REGULAR MONITORING OF THE ALUMINUM BOX CULVERT SHAPE IS NECESSARY DURING ALL BACKFILLING OF THE STRUCTURE.

INITIAL LIFTS OVER THE CROWN OF STRUCTURE AS

INDICATED BY SHADED AREA TO BE COMPACTED TO

OR WITH LIGHTWEIGHT(D-4 OR LIGHTER) EQUIPMENT.

REQUIRED DENSITY WITH HAND OPERATED EQUIPMENT

- PREVENT DISTORTION OF SHAPE AS NECESSARY BY VARYING COMPACTION METHODS AND EQUIPMENT
- TRENCH WIDTH OTHER THAN 3 FEET SHALL BE BY DIRECTION OF THE ENGINEER OF RECORD
- 5 SWITCH TO PLACING SELECT GRANULAR BACKFILL NEAR IN RADIAL LIFTS THE MIDDLE OF THE HAUNCH CURVE.

	Stone Fragment, Gravel and Sand		Silty or	Coarse Sand	
Plasticity Index	6 max.	6 max.	10 max.	10 max.	Non Plast
Liquid Limits			40 max.	41 min.	
At	terberg Limits fo	or Fraction Passin	g No. 40 (0.425 m	nm)	
No. 200 (0.075 mm)	15 max.	25 max.	35 max.	35 max.	10 max.
No. 40 (0.425 mm)	30 max.	50 max.		****	51 max.*
No. 10 (2.000 mm)	50 max.	2222	7 	1111	
Sieve Analysis Percent Passing	9				
GROUP CLASSIFICATION	A-1-a	A-1-b	A-2-4	A-2-5	A-3

3.0	ASSE	MBLY AND I		4					
erts"	3.1	Bolts and nuts shall conform to the requirements of ASTM A-307 and/or ASTM A-449. The box culvert shall be assembled in accordance with the plate layout drawings provided by the manufacturer and per the manufacturer's recommendations.							
		Bolts shall be lightened using an applied torque of between 100 and 150 ftlbs,							
	3.2	The box culvert shall be installed in accordance with the plans and specifications, the manufacturer's recommendations, and AASHTO Standard Specification for Highway Bridges - Section 26 Division II - Construction/AASHTO LRFD Bridge Construction Specifications - Section 26.							
	3,3	structurally plans. Poor	Trench excavation shall be made in embankment material that is structurally adequate. The trench width shall be shown on the plans. Poor quality in situ embankment material must be removed and replaced with suitable backfill as directed by the Engineer.						
e	3.4	soil-bearing accommoda	Aluminum Box Culvert designs require a minimum allowable soll-bearing pressure of 4,000 psf. Lower bearing capacities may be accommodated with a site specific design for an aluminum oundation or a concrete fooling.						
		If the engineer determines the natural foundation is inadequate to support the structure's backfill, the poor material shall be excavated, removed and replaced to a suitable depth with competent material. The specific depth of excavation required may be reduced by utilizing a geosynthetic reinforced foundation as designed by a qualified geotechnical engineer. For additional information contact your local Contech representative.							
	3.5	When a metal foundation is used, the soil bedding requires a minimum of 6 inches of loose granular material with a maximum particle size of one half the corrugation depth. The proper width of the bedding material required shall conform to the project plans and specifications.							
		Bedding preparation is critical to both structure performance and service life. The bedding should be constructed to uniform line and grade to avoid distortions that may create undesirable stresses in the structure and/or rapid deterioration of the roadway. The bed should be free of rock formations, protruding stones, frozen lumps, roots, and other foreign matter that may cause unequal settlement.							
and,	3.6	The structure shall be assembled in accordance with the Manufacturer's instructions. All plates shall be unloaded and handled with reasonable care. Plates shall not be rolled or dragged over gravel rock and shall be prevented from striking rock or other hard objects during placement in trench or on bedding.							
		When installed on a full invert or on flexible footing pads, assembly of the invert or footing pads shall start at the downstream end, Circumferential seam laps shall shingle over the top of the downstream plates as assembly progresses upstream. Whether the box culvert is installed on a concrete footing, full metal invert, or flexible footing pad, assembly of the structure shell shall start at the upstream end. Downstream rings of plates shall be assembled outside of the upstream rings (Circumferential seams are shingled downstream when viewed from the inside of the shell).							
	3.7	The structure shall be backfilled using clean well graded granular material that meets the requirements for soil classifications A-1, A-2-4, A-2-5, or A-3 modified per AASHTO M-145. See the structural plate backfill group classification table on this sheet.							
		Backfill must be placed symmetrically on each side of the structure in 8 inch loose lifts. Each lift shall be compacted to a minimum of 90 percent density per AASHTO T-180.							
	3.8	Standard highway loads that meet the permissible design load limits for an Aluminum Box Culvert are not allowed on the structure until it is backfilled completely and pavement is in place.							
		The addition of temporary soil for heavy construction loads is not feasible or permissible for Aluminum Box Culverts. By design, these structures are limited in the range of permissible fill heights and live loads.							
		highway live	load design I	that exceed that imits are not allo from the Engine	wed on Alumin		x		
	3.9	If an aluminum headwall and/or wingwall system is specified, the select granular structural backfill limits shall extend past the deadman anchor system. Contact the Engineer if stiff material or rock is encountered where the wingwalls and deadmen are to be installed.							
		INLET	OUTLET		CKNESS: .1				
SKE	W	0	0		ING RIB SP ING RIB TY				
BE\	/EL	0	0	NUMBER C	F STRUCT				
AT N	/IEA	DOW	PARK	STREET	PROJECT No. 576329	SEQ N		DATE: 1/10/2019	
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					SHEET NO.:			01151	
						9	OF	9	

