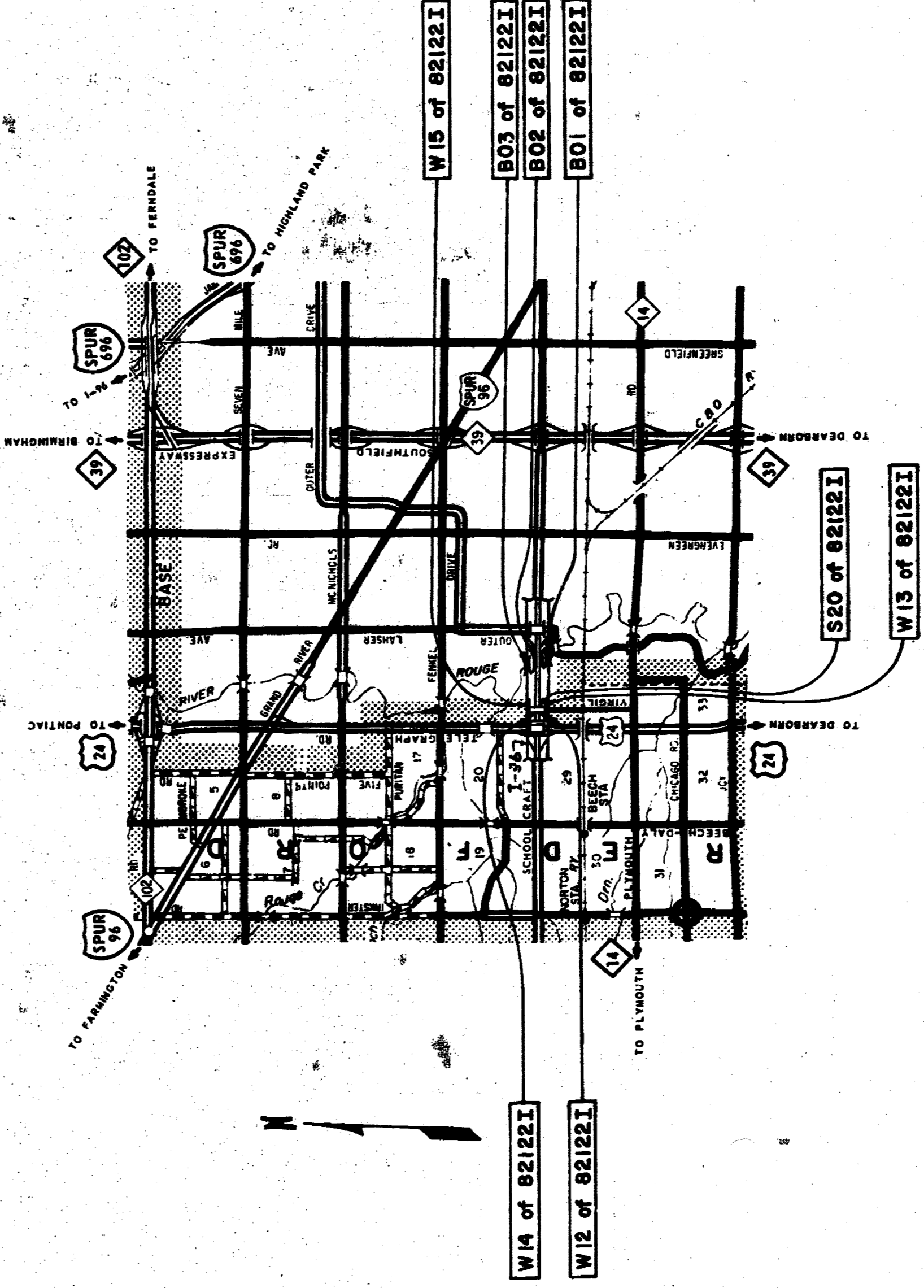


STATE OF MICHIGAN
DEPARTMENT OF STATE HIGHWAYS
PLANS OF PROPOSED BRIDGES & RETAINING WALLS
MICHIGAN PROJECT I-96-4(95) 220
STATE PROJECT I-82122-037 PART
BRIGHTON-DETROIT ROAD

WAYNE COUNTY
 CITY OF DETROIT
 JOB NUMBER 01250A



GENERAL NOTES

EXCEPT WHERE OTHERWISE INDICATED ON THESE PLANS OR IN THE PROPOSAL AND SUPPLEMENTAL SPECIFICATIONS CONTAINED THEREIN, ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE MICHIGAN DEPARTMENT OF STATE HIGHWAYS STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION 1970 EDITION.

THE DESIGN OF THIS STRUCTURE IS BASED ON THE MICHIGAN DEPARTMENT OF STATE HIGHWAYS SPECIFICATIONS FOR THE DESIGN OF HIGHWAY BRIDGES, 1958 EDITION AND CURRENT AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES EQUALS 1/1000 OF SPAN LENGTH AND 1/250 OF CANTILEVER ARM.

THE CHARACTER OF ALL MATERIALS AND THE EXTENT THEREOF AS SHOWN BY BORINGS HAS BEEN OBTAINED BY METHODS AND FROM SOURCES BELIEVED TO BE RELIABLE. THE EXACTNESS OF THIS INFORMATION IS, HOWEVER, IN NO CASE GUARANTEED.

ALL EXPOSED CONCRETE CORNERS SHOWN SQUARE ON THE PLANS SHALL BE BEVELED WITH 1/2" TRIANGULAR HOLDINGS EXCEPT AS OTHERWISE NOTED.

THE STATIONING AS SHOWN ON THESE PLANS FOR THE INTERSECTION OF THE CENTERLINE OF BRIDGE AND CHANNEL CENTERLINE IS BELIEVED TO BE CORRECT. IT SHALL, HOWEVER, BE CHECKED AT THE TIME OF STARTING CONSTRUCTION AND TO THE STATIONING OFFICE AT LANSING AND THE STRUCTURE SHALL BE STAKED OUT USING THE ACTUAL INTERSECTION OF THE CENTERLINE OF BRIDGE AND CENTERLINE AS THE CONTROL POINT.

THE GRADES AND STRESSES OF THE STRUCTURAL MATERIALS USED IN THIS STRUCTURE ARE AS FOLLOWS:

CONCRETE: GRADE A..... $f_c = 3,000$ psi
 STEEL REINFORCEMENT..... $f_s = 20,000$ psi

STRUCTURAL STEEL: A36..... $f_y = 20,000$ psi
 A388..... $f_y = 27,000$ psi

6-75
CONSTRUCTED
 AS PER PLANS

| | | | |
|--|------------------------------|--|---------|
| ITEM NO. | | CONTRACT FOR BRIDGES AND RETAINING WALLS | |
| CHECKED | | APPROVALS | |
| RECOMMENDED FOR APPROVAL | ENGINEER - DESIGN SECTION II | DATE | 5-17-70 |
| RECOMMENDED FOR APPROVAL | ENGINEER OF DESIGN | DATE | 5-25-70 |
| RECOMMENDED FOR APPROVAL | TRAFFIC DIVISION | DATE | 4-24-70 |
| RECOMMENDED FOR APPROVAL | CONSTRUCTION DIVISION | DATE | 6-24-70 |
| CHIEF - BUREAU OF ENGINEERING DEPARTMENT OF STATE HIGHWAYS HENRIK E. STAPSETH - STATE HIGHWAY DIRECTOR | | | |
| APPROVED BY | | DATE | |
| PLANS PREPARED BY | | DATE | |
| U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION APPROVED BUREAU OF PUBLIC ROADS | | | |
| FISHER AILUNI SQUAD 525 W-3 B01 W-4 B-802 of 82122I W-5 B-805 | | | |
| SHEET NO. | | TITLE | |
| E-4-A-163 | | STRUCTURE GUARD RAIL ANCHORAGE TYPE A | |
| STATE PROJECT NO. | | FEDERAL PROJECT NO. | |
| I 82122-037I-96- | | 1 | |

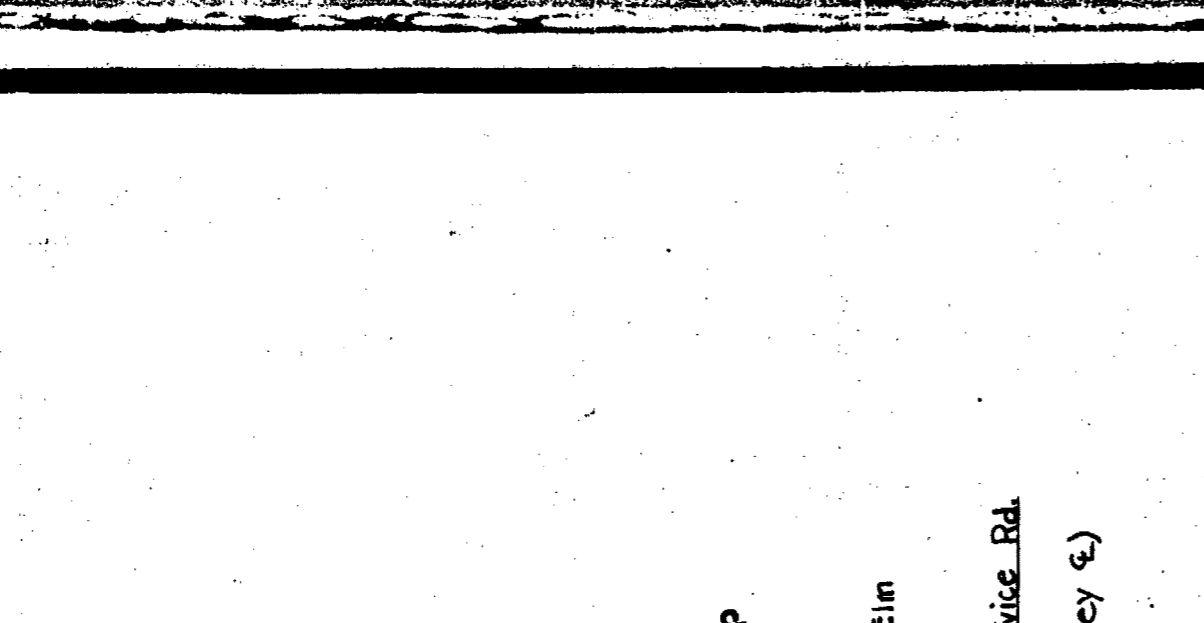
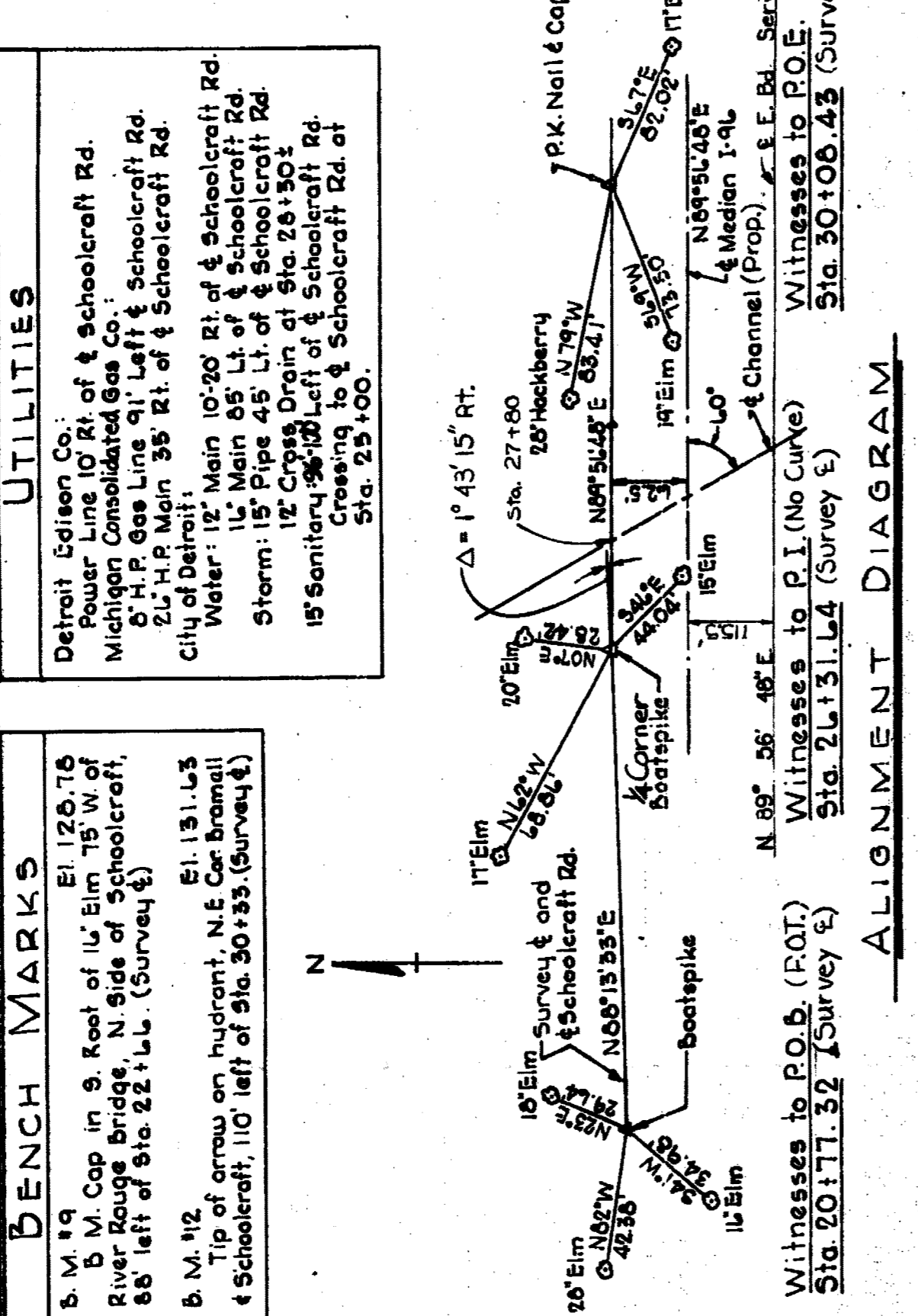
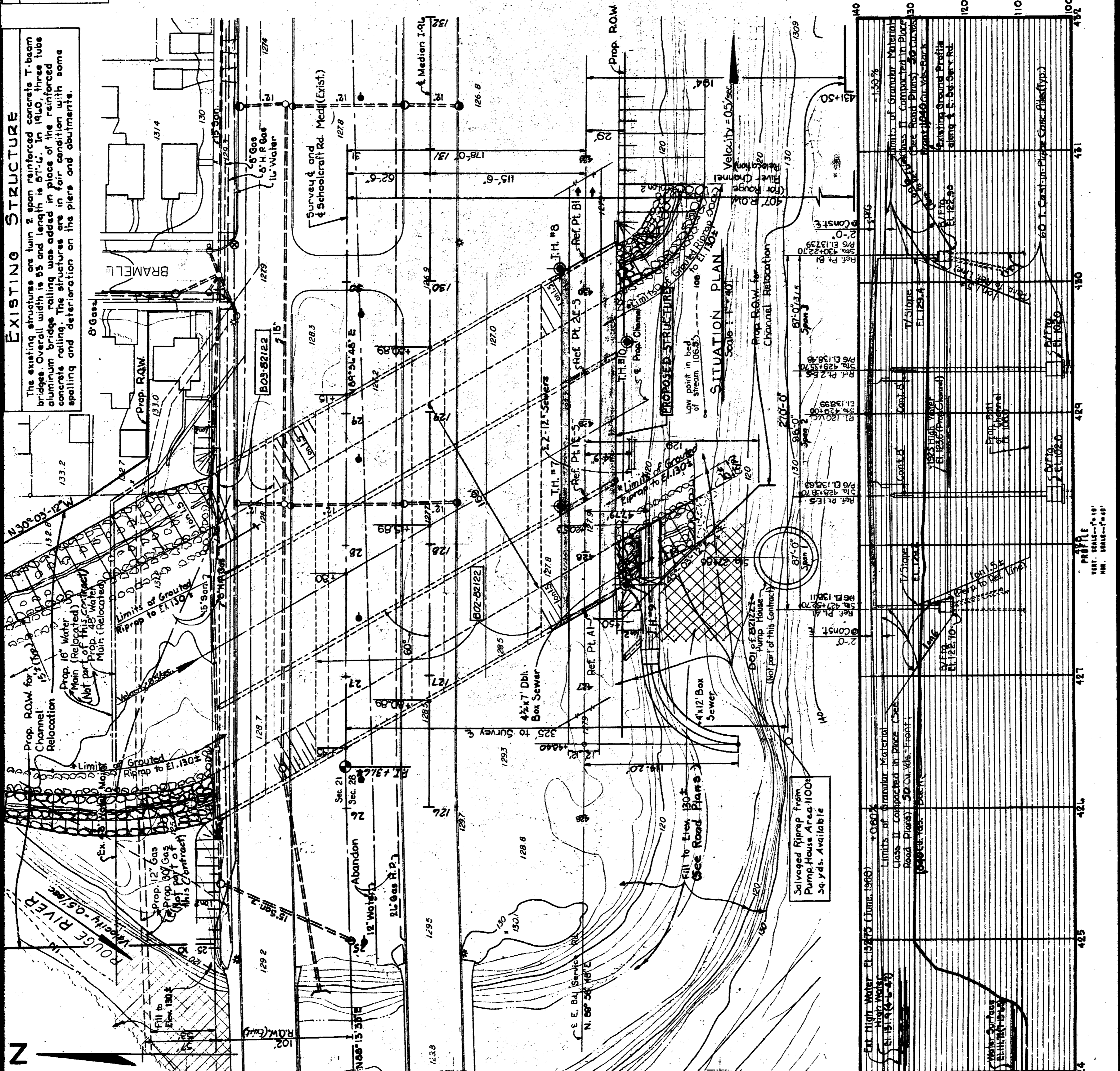
STANDARD PLANS NOT TO BE PRINTED

NOTE
 WHERE THE FOLLOWING ITEMS ARE CALLED FOR ON THE PLANS THEY ARE TO BE CONSTRUCTED ACCORDING TO THE STANDARD PLAN GIVEN BELOW OPPOSITE EACH ITEM UNLESS OTHERWISE INDICATED.

| SHEET NO. | TITLE |
|-----------|---|
| R11V | BRIDGE RAILING, DRAIN CASTING, BAR CHAIR, MOLDING AND BEVEL DETAILS |
| R13 | BRIDGE RAILING, DRAIN CASTING, BAR CHAIR, MOLDING AND BEVEL DETAILS |
| R15B | BRIDGE RAILING, DRAIN CASTING, BAR CHAIR, MOLDING AND BEVEL DETAILS |
| R16B | BRIDGE RAILING, DRAIN CASTING, BAR CHAIR, MOLDING AND BEVEL DETAILS |

JOB NUMBER 01250A

PROJECT NO. I-82122-037
 01250A B0105



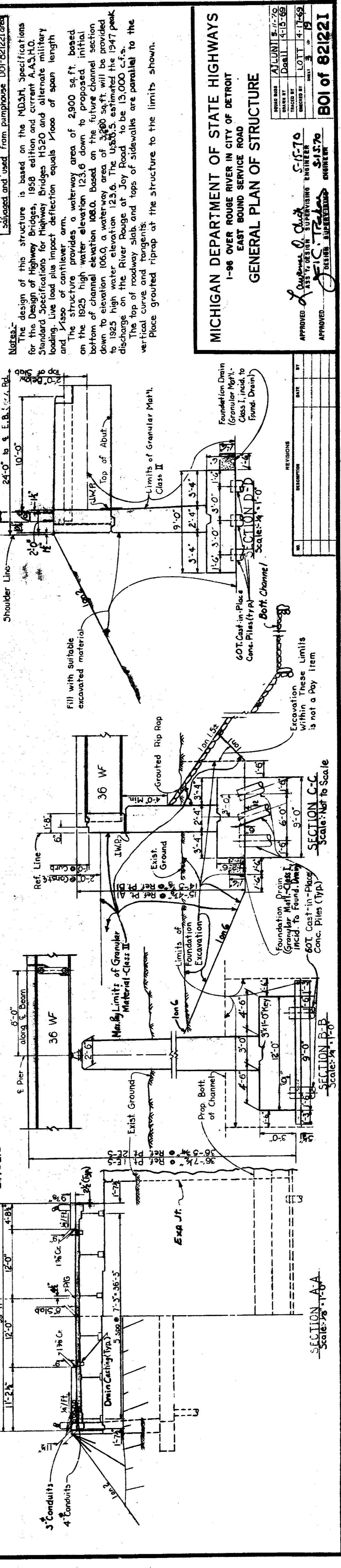
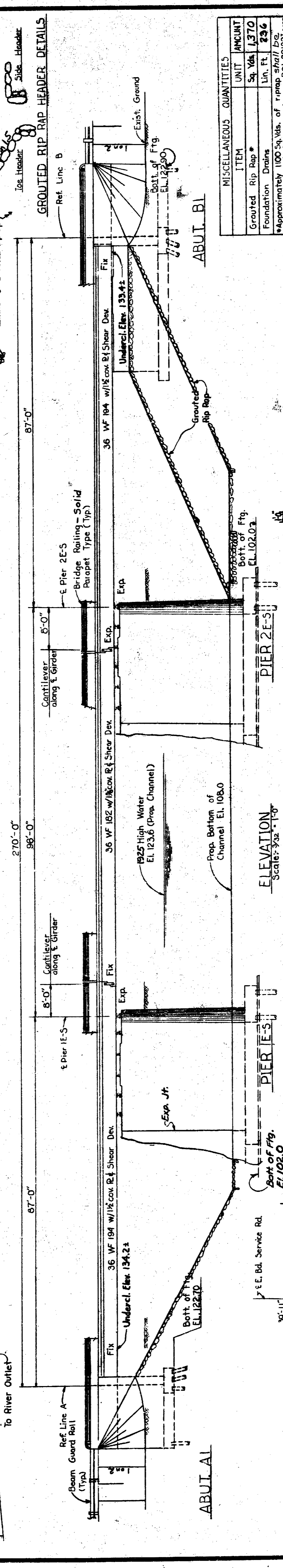
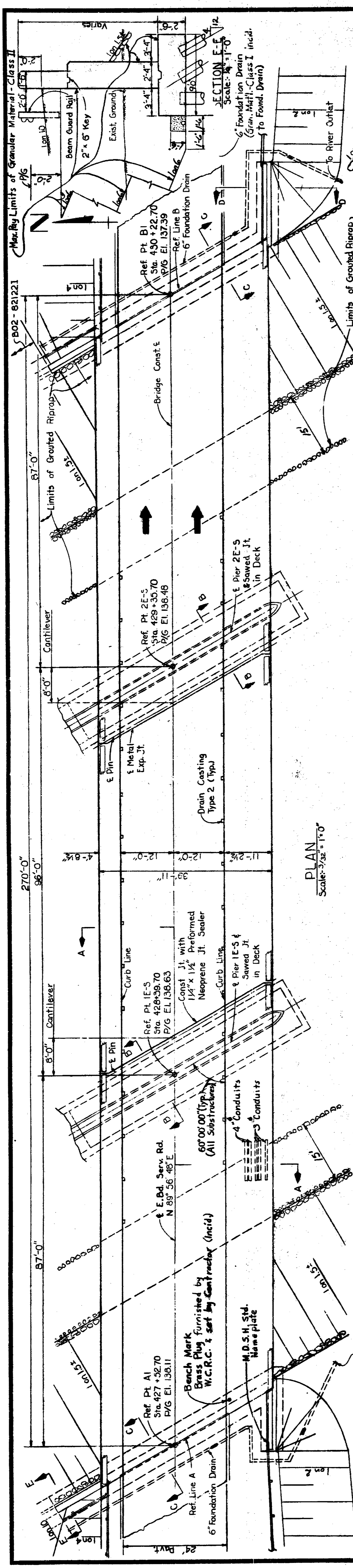
CONSTRUCTION SEQUENCE FOR STRUCTURES
BOI of 821221 & BO2 of 821221 are to be constructed and opened to traffic prior to construction of BO3 of 821221. (See Construction Stages in Road Plans.)

NOTES
This work covered by these plans includes construction of the proposed bridge replacing riprap to the limits shown. All other work is included in the Road Plans which are a part of this contract.
The contractor shall locate all active underground utilities prior to starting work, and shall conduct his operations in such a manner as to insure that those utilities not requiring relocation will not be disturbed.
Datum refers to Detroit Datum, Elevation 102.00 - U.S.G.S. Elevation 578.76
Topography shown herein represents conditions prior to construction of BOI of 821221 and is altered as shown on the attached sheet for BOI of 821221.
*For grooved riprap use broken concrete salvaged from the existing bridges & existing conc. pav't.
Log of borings included with Plans of BO2 of 821221.

STATE OF MICHIGAN
Department of State Highways
EAST BOUND SERVICE ROAD
GENERAL PLAN OF SITE

| | | |
|----------|------|----------|
| DATE | BY | REVISION |
| 11/21/78 | J.A. | 1.00 |
| 12/12/78 | J.A. | 1.01 |
| 1/15/79 | J.A. | 1.02 |
| 2/15/79 | J.A. | 1.03 |
| 3/15/79 | J.A. | 1.04 |
| 4/15/79 | J.A. | 1.05 |
| 5/15/79 | J.A. | 1.06 |
| 6/15/79 | J.A. | 1.07 |
| 7/15/79 | J.A. | 1.08 |
| 8/15/79 | J.A. | 1.09 |
| 9/15/79 | J.A. | 1.10 |
| 10/15/79 | J.A. | 1.11 |
| 11/15/79 | J.A. | 1.12 |
| 12/15/79 | J.A. | 1.13 |
| 1/15/80 | J.A. | 1.14 |
| 2/15/80 | J.A. | 1.15 |
| 3/15/80 | J.A. | 1.16 |
| 4/15/80 | J.A. | 1.17 |
| 5/15/80 | J.A. | 1.18 |
| 6/15/80 | J.A. | 1.19 |
| 7/15/80 | J.A. | 1.20 |
| 8/15/80 | J.A. | 1.21 |
| 9/15/80 | J.A. | 1.22 |
| 10/15/80 | J.A. | 1.23 |
| 11/15/80 | J.A. | 1.24 |
| 12/15/80 | J.A. | 1.25 |

APPROVED: J.A. J.A. 5-5-78
PROJECT SUPERVISOR
APPROVED: J.A. J.A. 5-5-78
DESIGN SUPERVISOR



MISCELLANEOUS QUANTITIES

| ITEM | UNIT | AMOUNT |
|-------------------|----------|--------|
| Grouted Rip Rap | Sq. Yds. | 1,370 |
| Foundation Drains | Lin. Ft. | 236 |

*Approximately 100 sq. yds. of riprap shall be salvaged and used from pump-house DOI-221222 area

Notes:
 The design of this structure is based on the MDSM, Specifications for the Design of Highway Bridges, 1958 edition and current A.A.S.H.O. Standard Specifications for Highway Bridges, H520 and alternate military loading. Live load plus impact deflection equals $1/1000$ of span length and $1/800$ of cantilever arm.
 The structure provides a waterway area of 2,900 sq. ft. based on the 1925 high water elevation 123.6 down to proposed initial bottom of channel elevation 108.0. Based on the future channel section down to elevation 106.0, a waterway area of 3,200 sq. ft. will be provided by 1925 high water elevation 23.6. The United States estimated the 1947 peak discharge of the River Rouge at Joy Road to be 13,000 c.f.s.
 The top of roadway slab and tops of sidewalks are parallel to the vertical curve and tangents.
 Place grouted riprap at the structure to the limits shown.

MICHIGAN DEPARTMENT OF STATE HIGHWAYS
 1-96 OVER ROUGE RIVER IN CITY OF DETROIT
 EAST BOUND SERVICE ROAD
GENERAL PLAN OF STRUCTURE

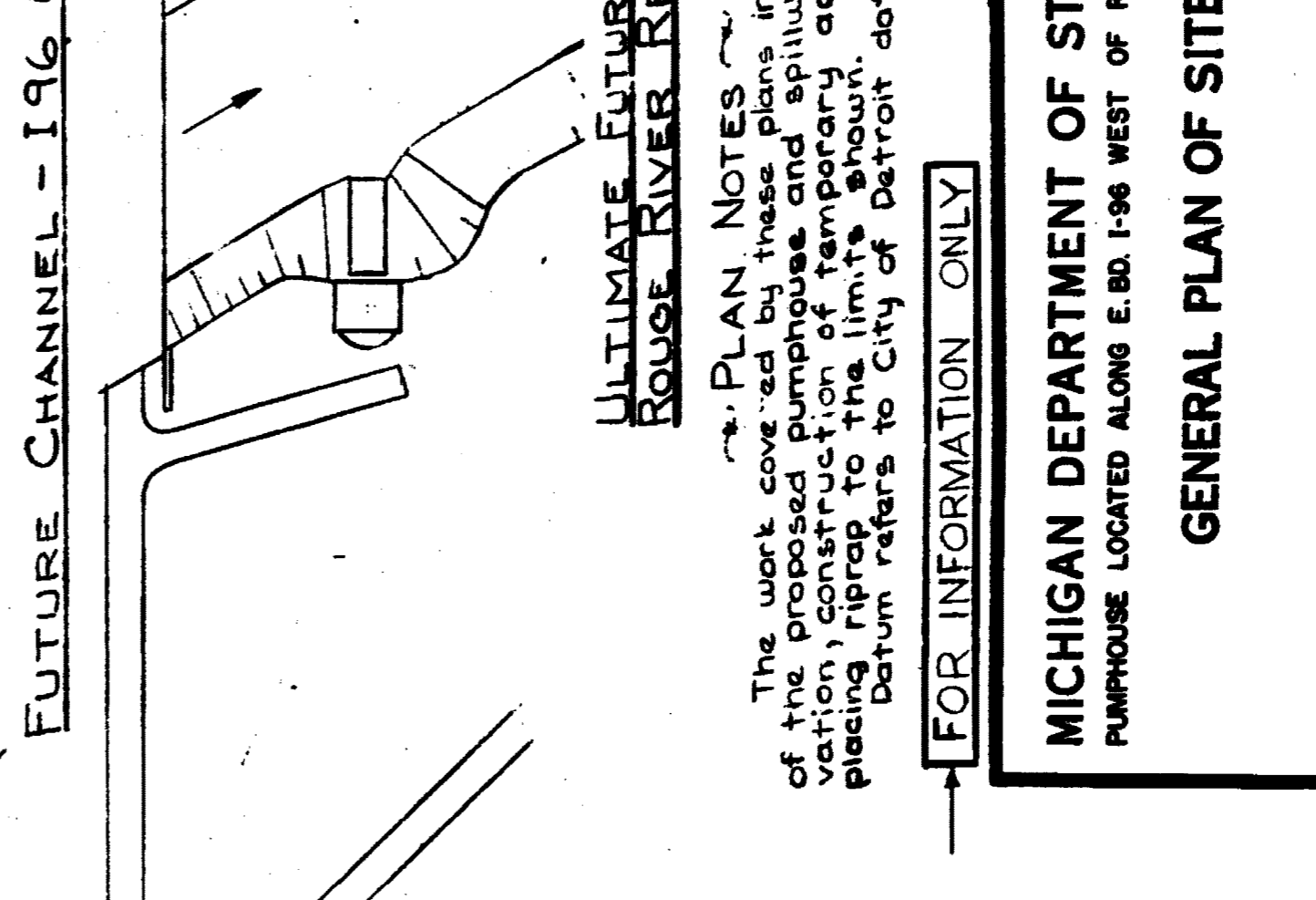
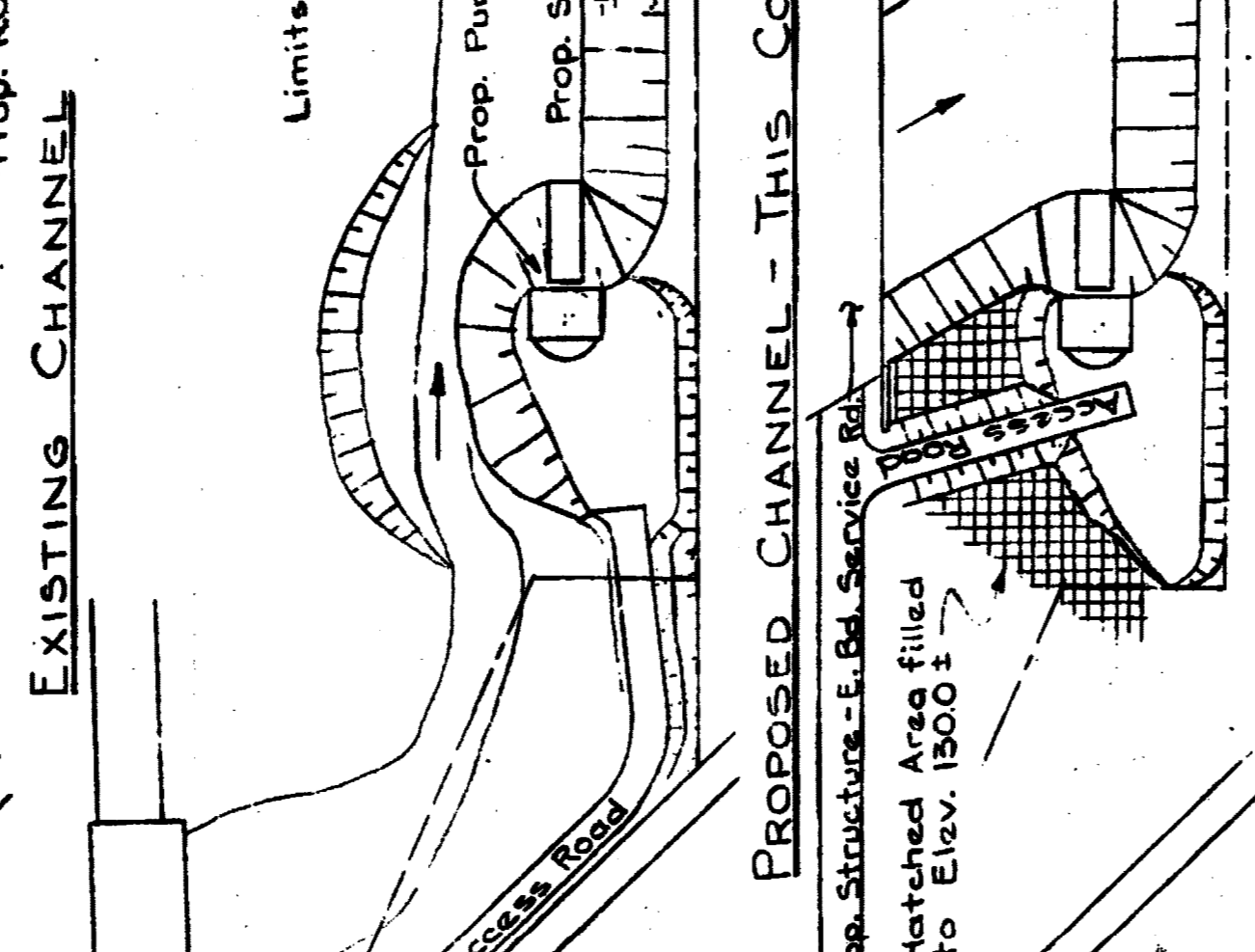
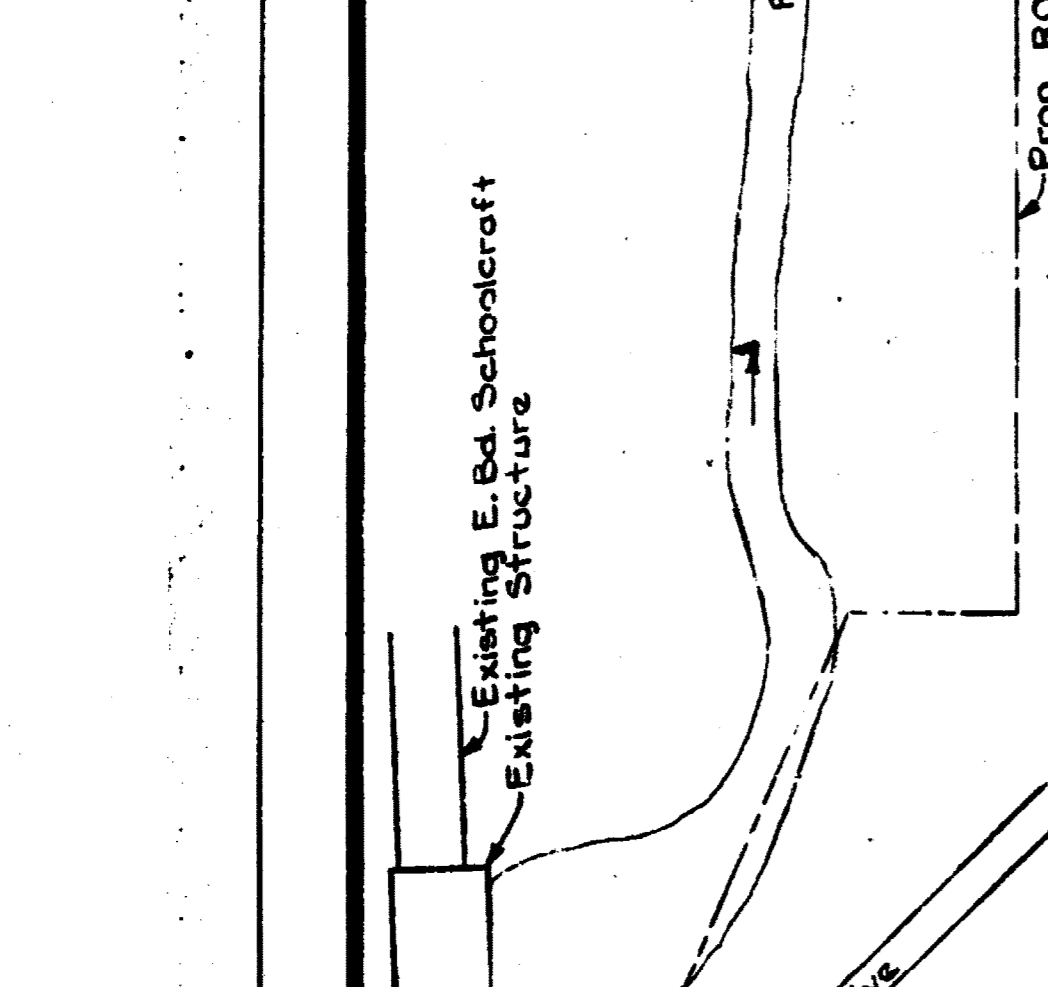
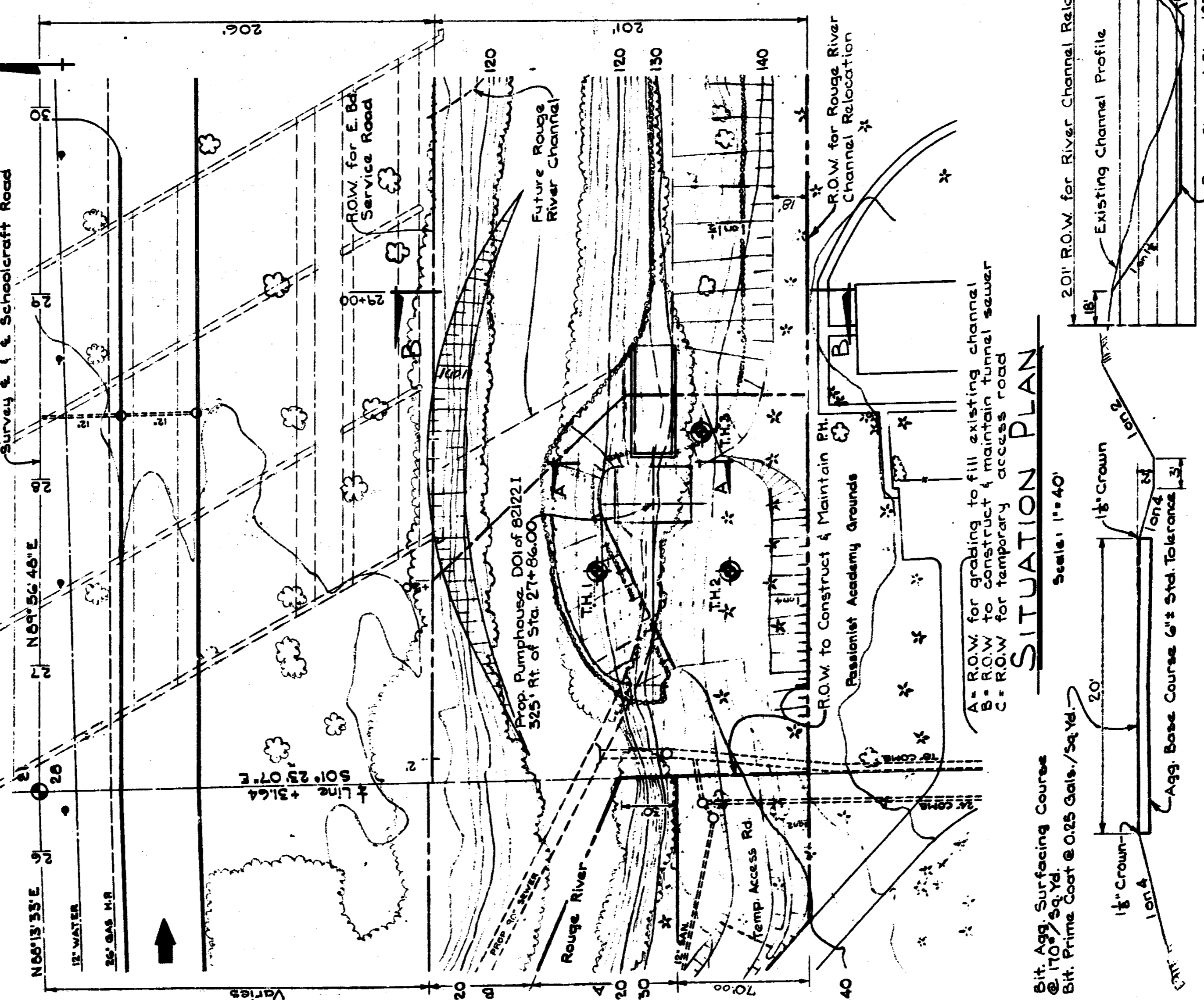
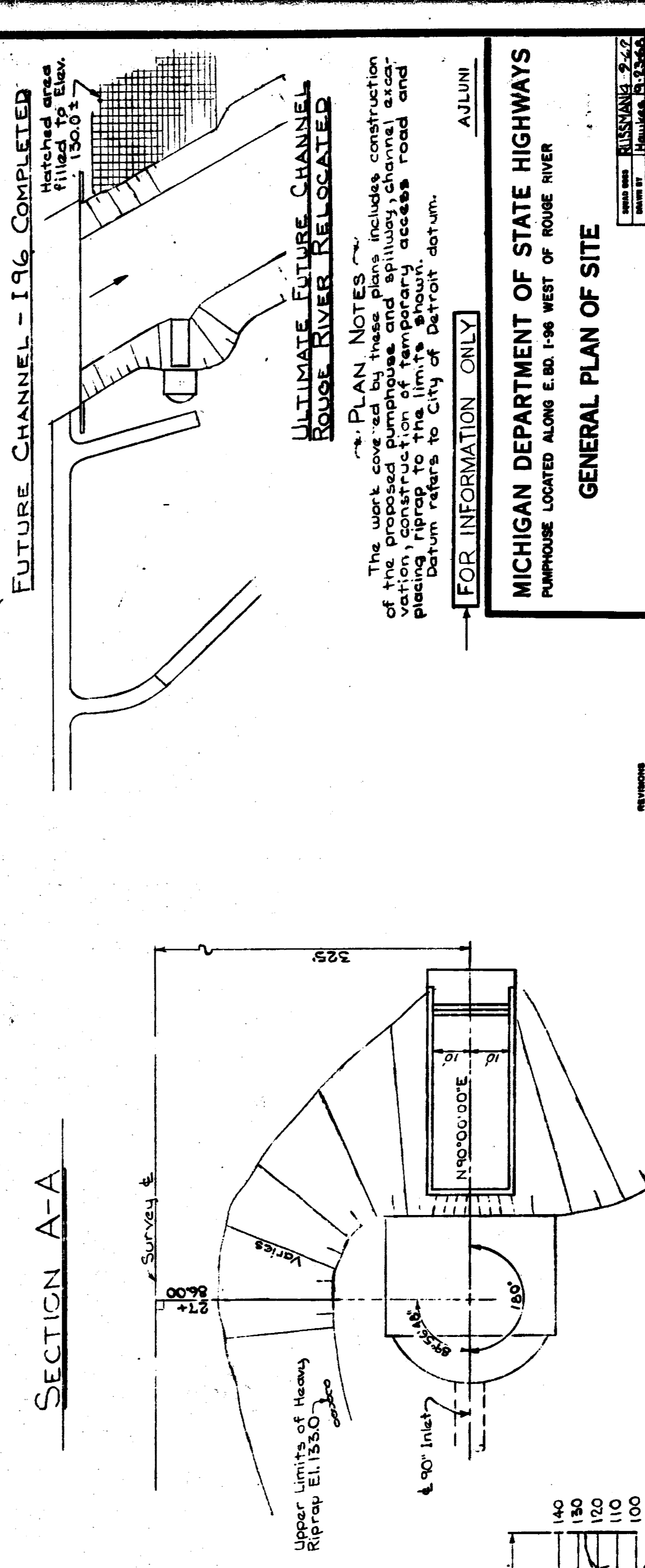
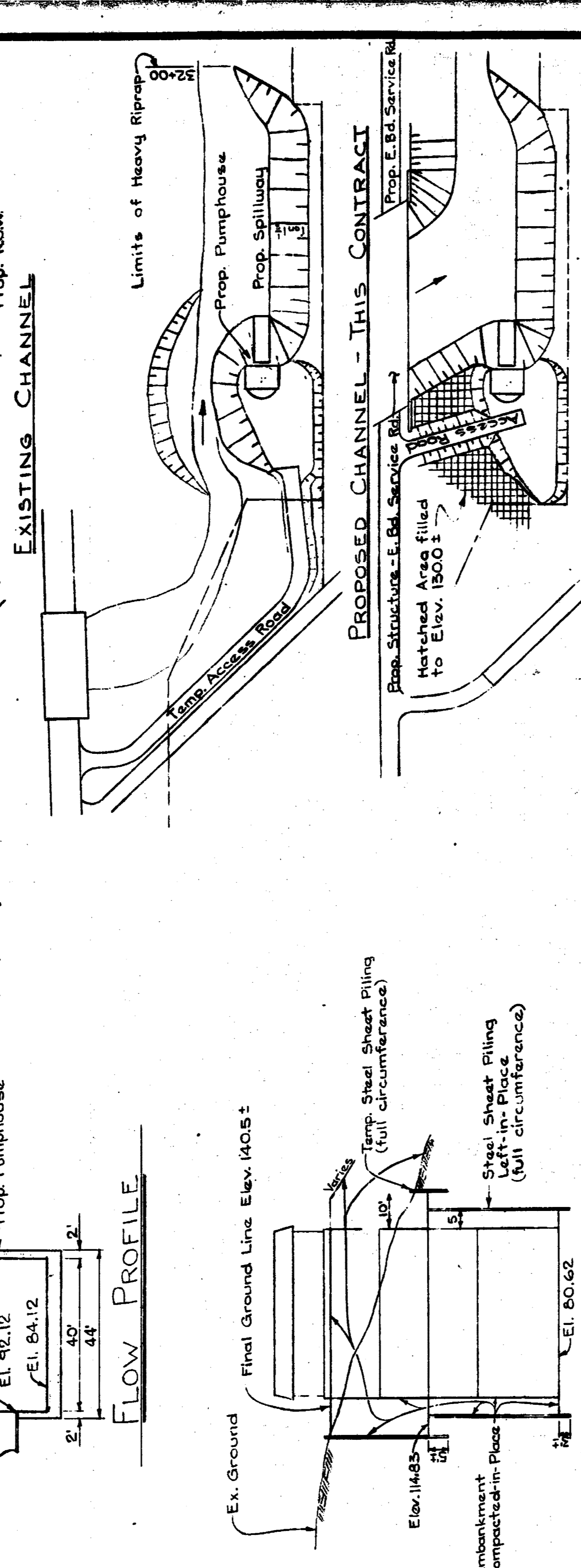
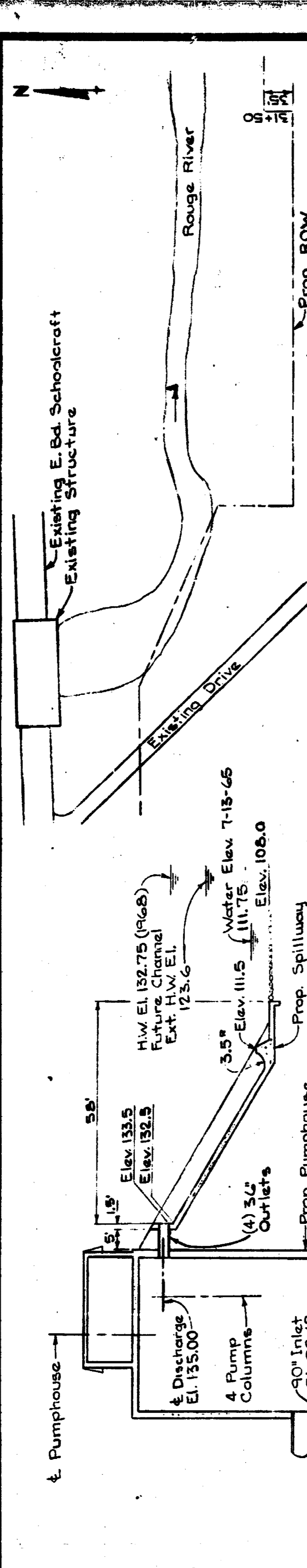
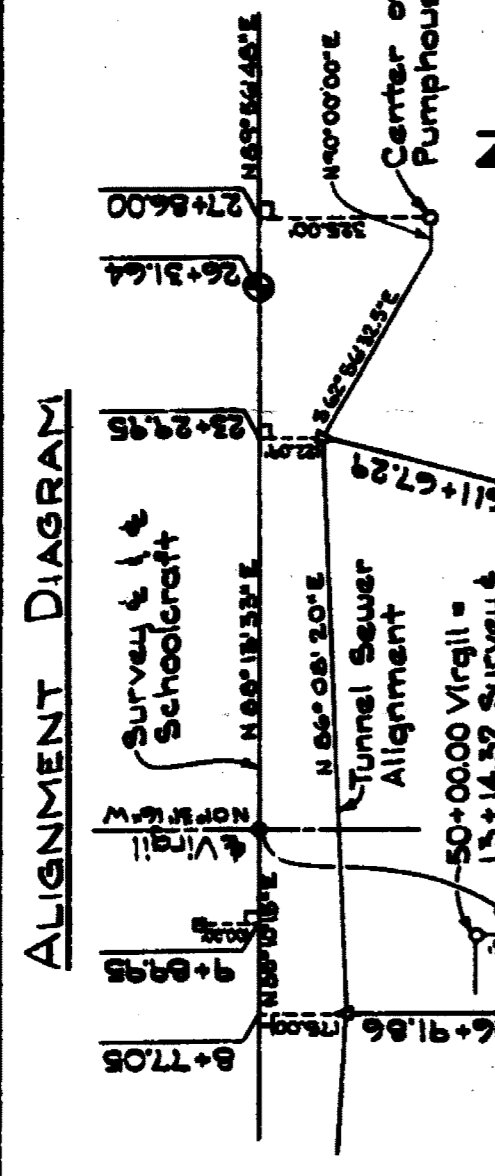
BOI of 821221

| NO. | DATE | REVISIONS |
|-----|--------------|-----------|
| 1 | JULY 19 1957 | REVISION |
| 2 | AUG 22 1957 | REVISION |
| 3 | SEPT 11 1957 | REVISION |
| 4 | NOV 20 1957 | REVISION |
| 5 | DEC 10 1957 | REVISION |

APPROVED: *[Signature]* 5-15-70
 ASSIST. TO DESIGN SUPERVISOR ENGINEER
 APPROVED: *[Signature]* 5-15-70
 DESIGN SUPERVISOR ENGINEER

| BENCH MARKS | |
|-------------------------------|------------|
| B.M. #10 | El. 129.44 |
| N.E. Cor. River Rouge Bridge | |
| City B.M. #120-255 | |
| Recorded El. 133.077 | |
| B.M. #11 | El. 126.69 |
| 4" Appl. 100' N. of | |
| N.E. Cor. River Rouge Bridge | |
| City B.M. #120-255 | |
| Recorded El. 133.077 | |
| B.M. #12 | El. 131.63 |
| Tip of arrow on hydrant | |
| N.E. corner Bramell & School- | |
| craft 110' W. of Sta. 50+55 | |

| ALIGNMENT | |
|------------------------------|----------------------|
| P1- Sta. 96+31.64 | Defl. 1° 43' 15" Rt. |
| No. Curve | |
| WITNESSES | |
| Application 100' N. of | |
| N.E. Cor. River Rouge Bridge | |
| City B.M. #120-255 | |
| Recorded El. 133.077 | |
| N130°W-44.09'-Cor. Bldg. | |
| N130°W-27.87'-Cor. Bldg. | |
| Drill hole in chisel sq. | |



**ULTIMATE FUTURE CHANNEL
ROUGE RIVER RELOCATED**

PLAN NOTES
The work covered by these plans includes construction of the proposed pump house and spillway, channel excavation, construction of temporary access road and placing riprap to the limits shown. Datum refers to City of Detroit datum.

FOR INFORMATION ONLY AJLJN1

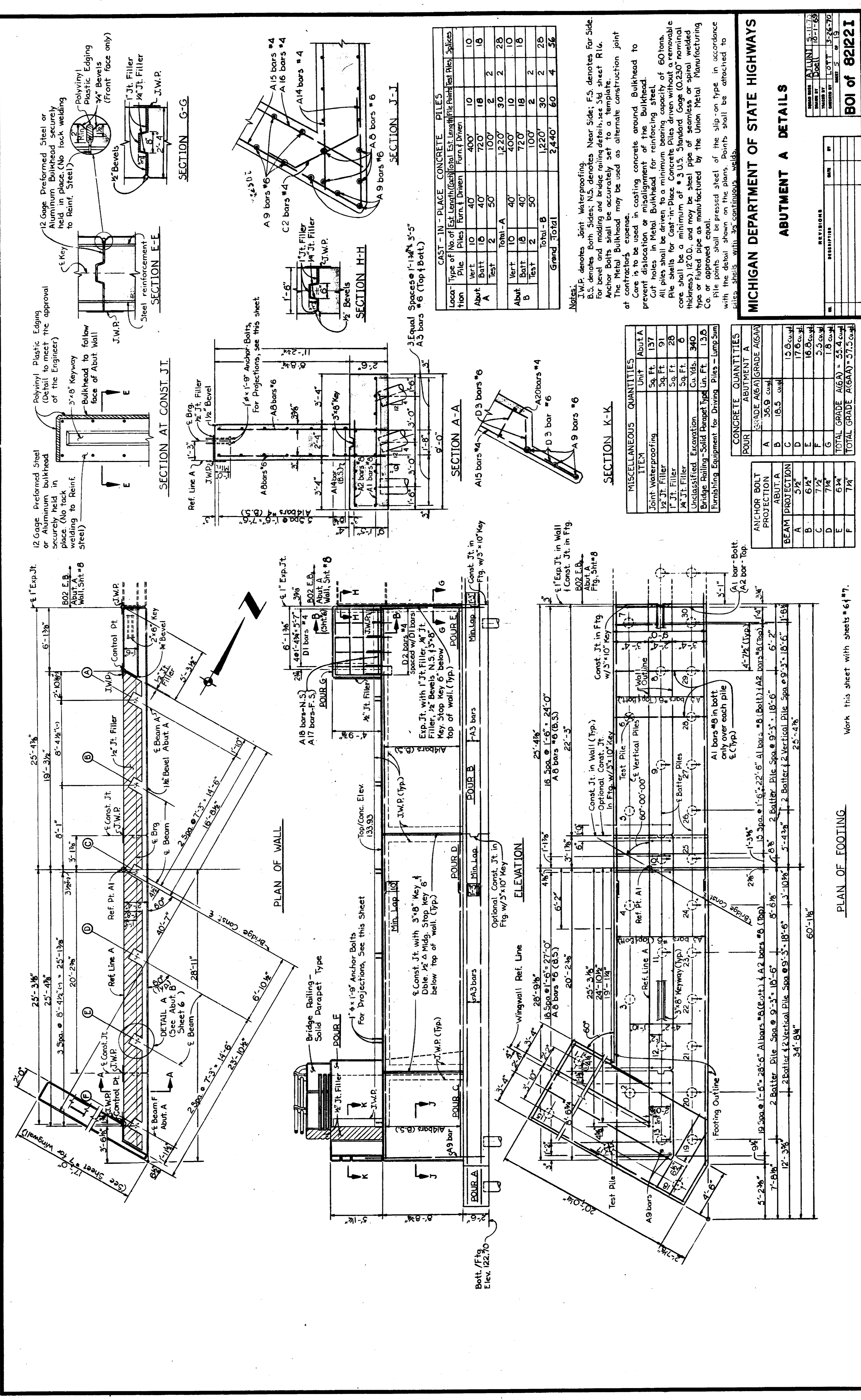
MICHIGAN DEPARTMENT OF STATE HIGHWAYS
PUMP HOUSE LOCATED ALONG E. BD. 1-96 WEST OF ROUGE RIVER

GENERAL PLAN OF SITE

| | |
|-----------------------------------|-------------|
| DESIGNED BY | REVISIONS |
| DRAWN BY | NO. |
| CHECKED BY | DATE |
| APPROVED | DESCRIPTION |
| ASST. DESIGN SUPERVISING ENGINEER | |
| DESIGN SUPERVISOR ENGINEER | |

DOI of 82122 I

012300



CAST-IN-PLACE CONCRETE PILES

| Location | Type of Pile | No. of Piles | Length (Feet) | Volume (Cu Yds) | Form (Sq Ft) | Turn (Hrs) | Driven (Hrs) | Splices |
|-------------|--------------|--------------|---------------|-----------------|--------------|------------|--------------|---------|
| Abut. A | Vert. | 10 | 40' | 400' | 10 | | | 10 |
| | Batt. | 10 | 72' | 720' | 18 | | | 18 |
| | Test | 2 | 50' | 100' | 2 | | | 2 |
| Abut. B | Vert. | 10 | 40' | 400' | 10 | | | 10 |
| | Batt. | 10 | 72' | 720' | 18 | | | 18 |
| | Test | 2 | 50' | 100' | 2 | | | 2 |
| Total - A | | | | 1,220' | 30 | | | 28 |
| Total - B | | | | 1,220' | 30 | | | 28 |
| Grand Total | | | | 2,440' | 60 | | | 56 |

Notes:
 J.W.P. denotes Joint Waterproofing.
 B.S. denotes Both Sides; N.S. denotes Near Side; F.S. denotes Far Side.
 For bevel and mending and bridge rolling details, see 3rd sheet R16.
 Anchor Bolts shall be accurately set to a template.
 The Metal Bulkhead may be used as alternate construction joint of contractor's expense.
 Core is to be used in casting concrete around Bulkhead to prevent displacement or misalignment of the Bulkhead.
 Cut holes in Metal Bulkhead for reinforcing steel.
 All piles shall be driven to a minimum bearing capacity of 60 tons.
 Pile shafts for cast-in-place concrete piles driven without a removable core shall be minimum of #3 U.S. Standard Gage (0.230" nominal thickness) 12" O.D. and may be steel pipe of seamless or spiral welded type or tubed pipe as manufactured by the Union Metal Manufacturing Co. or approved equal.
 Pile points shall be pressed steel of the slip-on type in accordance with the detail shown on the plans. Points shall be attached to piles shafts with continuous welds.

MISCELLANEOUS QUANTITIES

| ITEM | Unit | Abut. A | Abut. B |
|---|----------|---------|---------|
| Joint Waterproofing | Sq. Ft. | 137 | 91 |
| 1/2" J.T. Filler | Sq. Ft. | 28 | 28 |
| 1/4" J.T. Filler | Sq. Ft. | 0 | 0 |
| Unclassified Excavation | Cu. Yds. | 340 | 340 |
| Bridge Rolling - Solid Parapet Type | Ln. Ft. | 13.8 | 13.8 |
| Furnishing Equipment for Driving Piles - Lump Sum | | | |

CONCRETE QUANTITIES

| POUR | ABUTMENT A | ABUTMENT B |
|--------------------------------|------------|------------|
| A | 36.9 cu yd | 36.9 cu yd |
| B | 18.5 cu yd | 18.5 cu yd |
| C | | 15.8 cu yd |
| D | | 17.6 cu yd |
| E | | 16.8 cu yd |
| F | | 5.5 cu yd |
| G | | 1.8 cu yd |
| TOTAL GRADE (AGA) = 53.4 cu yd | | |
| TOTAL GRADE (AGB) = 57.5 cu yd | | |

MICHIGAN DEPARTMENT OF STATE HIGHWAYS

ABUTMENT A DETAILS

BOI of 821221

012300

REVISIONS

| NO. | DESCRIPTION | DATE | BY |
|-----|-------------|---------|--------|
| 1 | AS SHOWN | 5-11-70 | ALUMNI |
| 2 | AS SHOWN | 10-1-70 | ALUMNI |
| 3 | AS SHOWN | 5-26-70 | ALUMNI |
| 4 | AS SHOWN | 5-19-70 | ALUMNI |

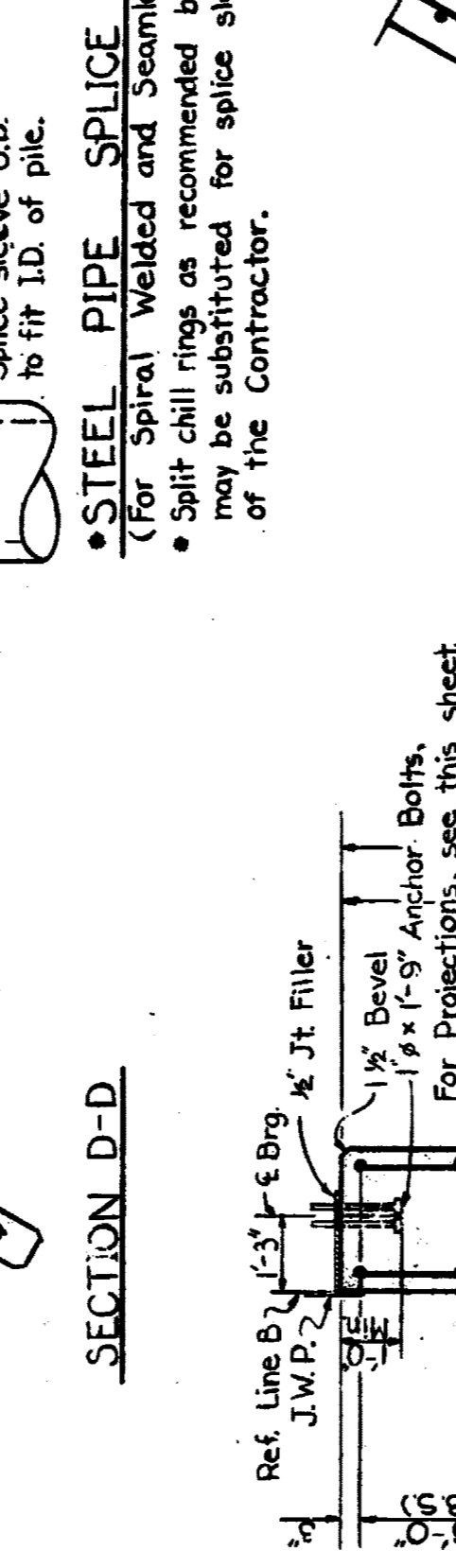
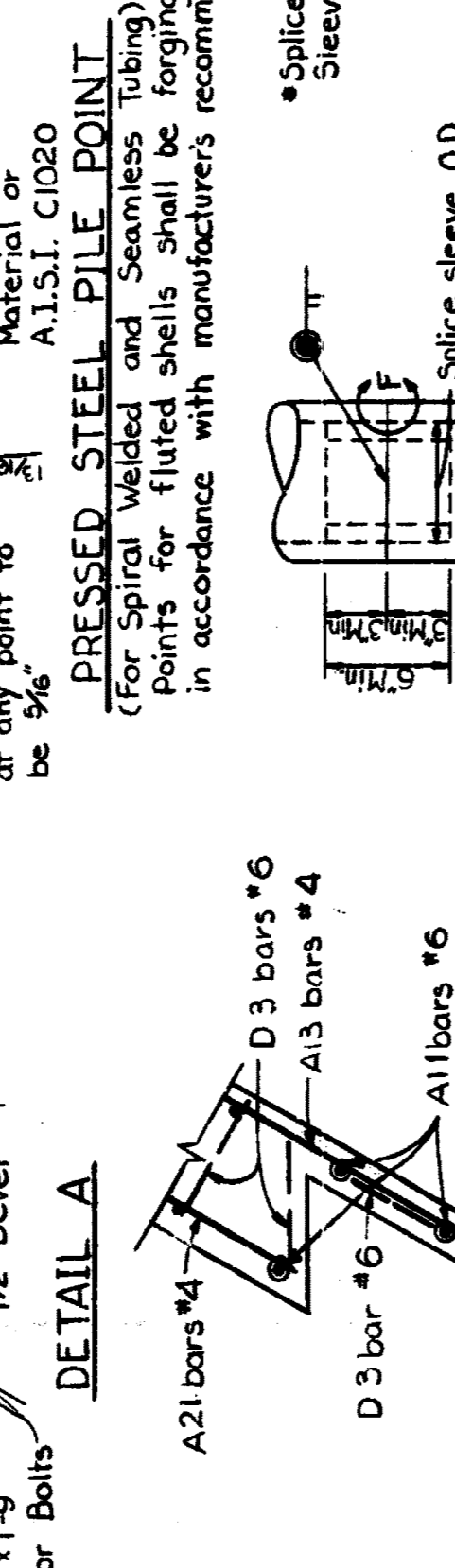
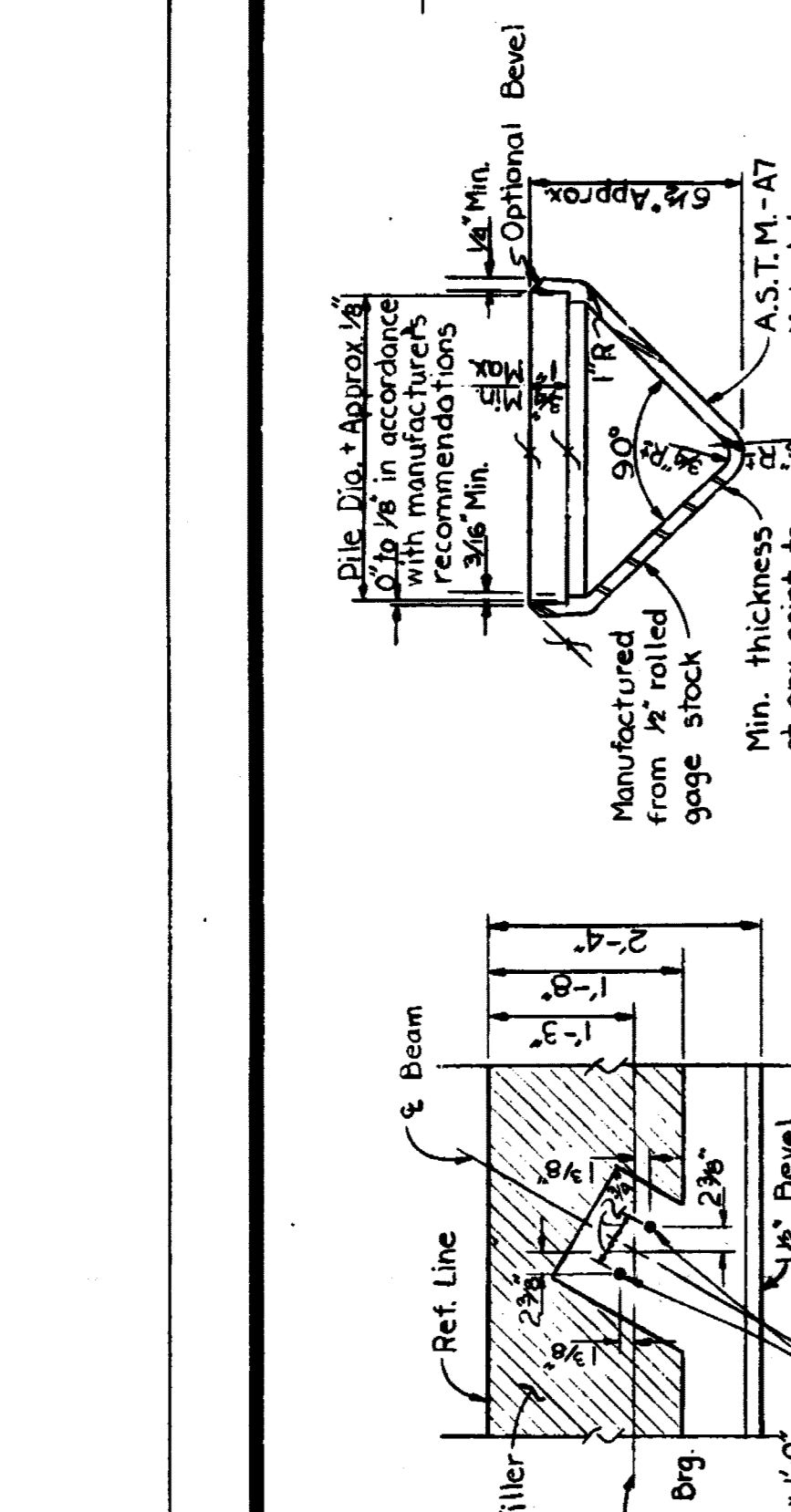
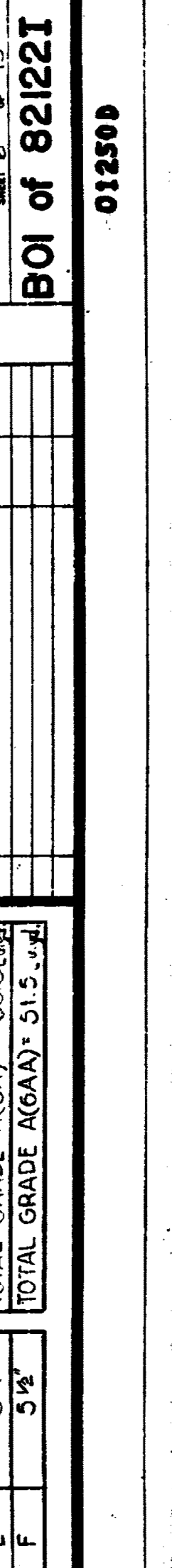
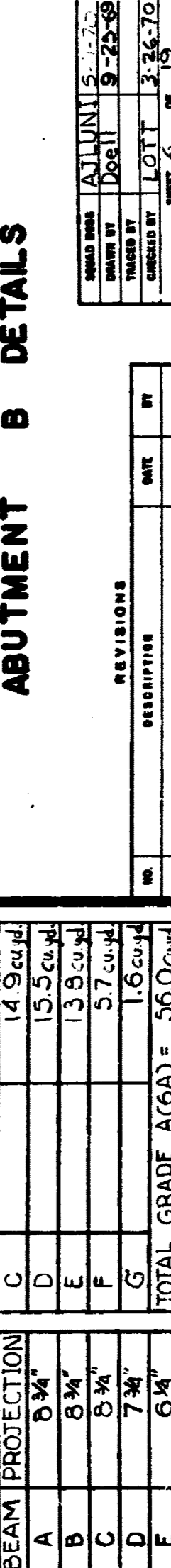
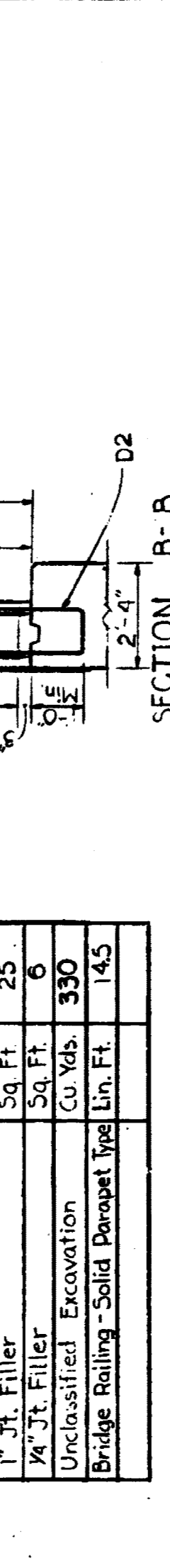
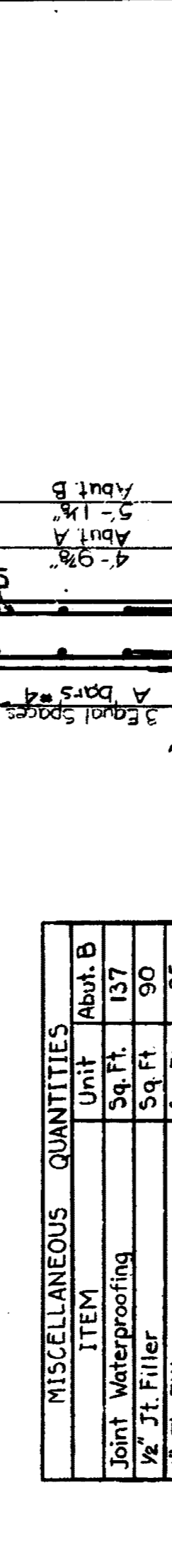
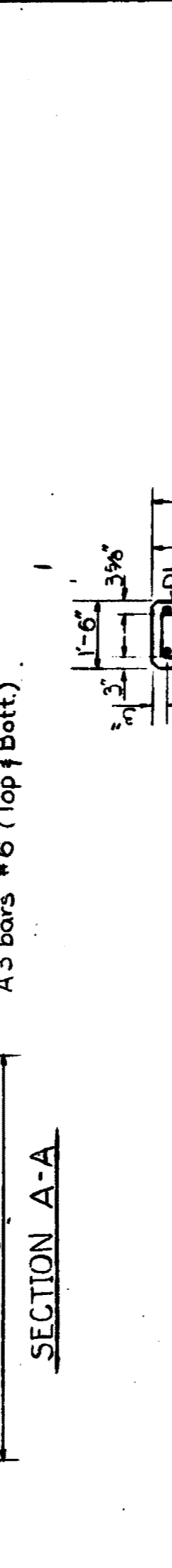
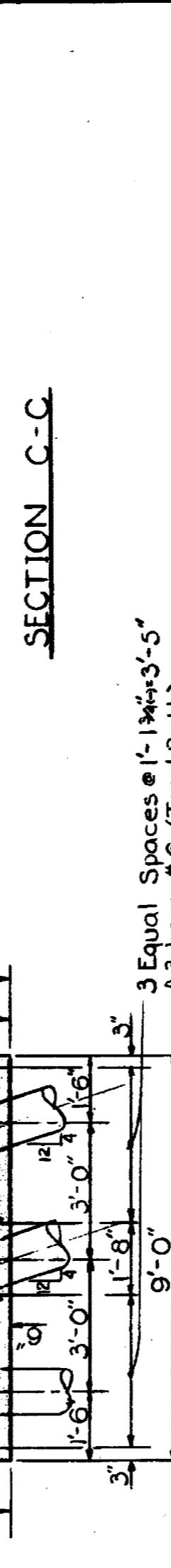
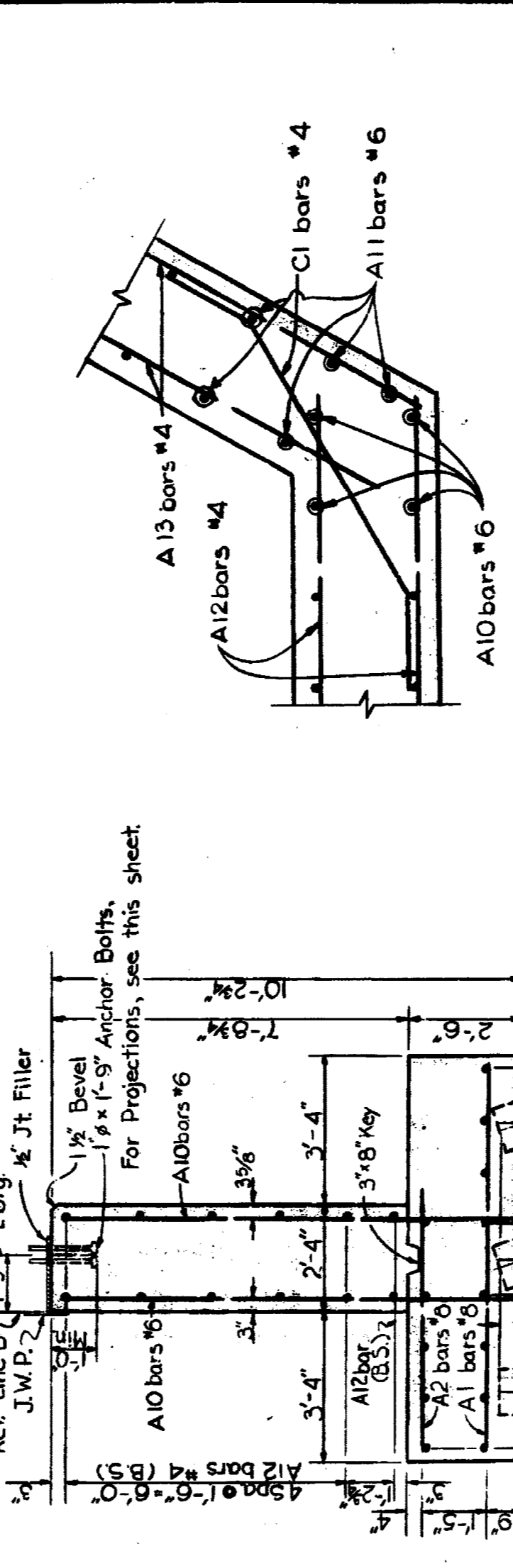
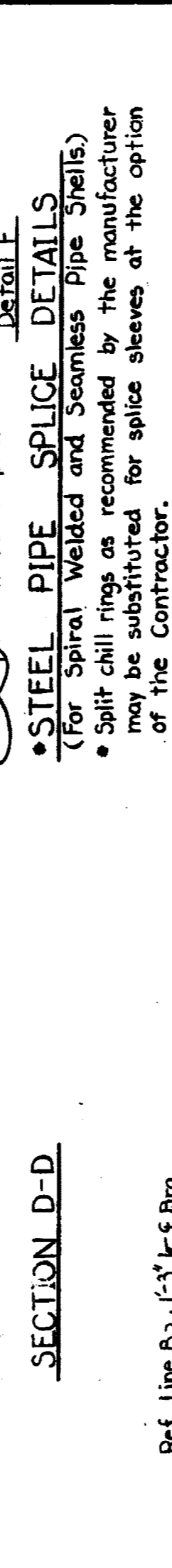
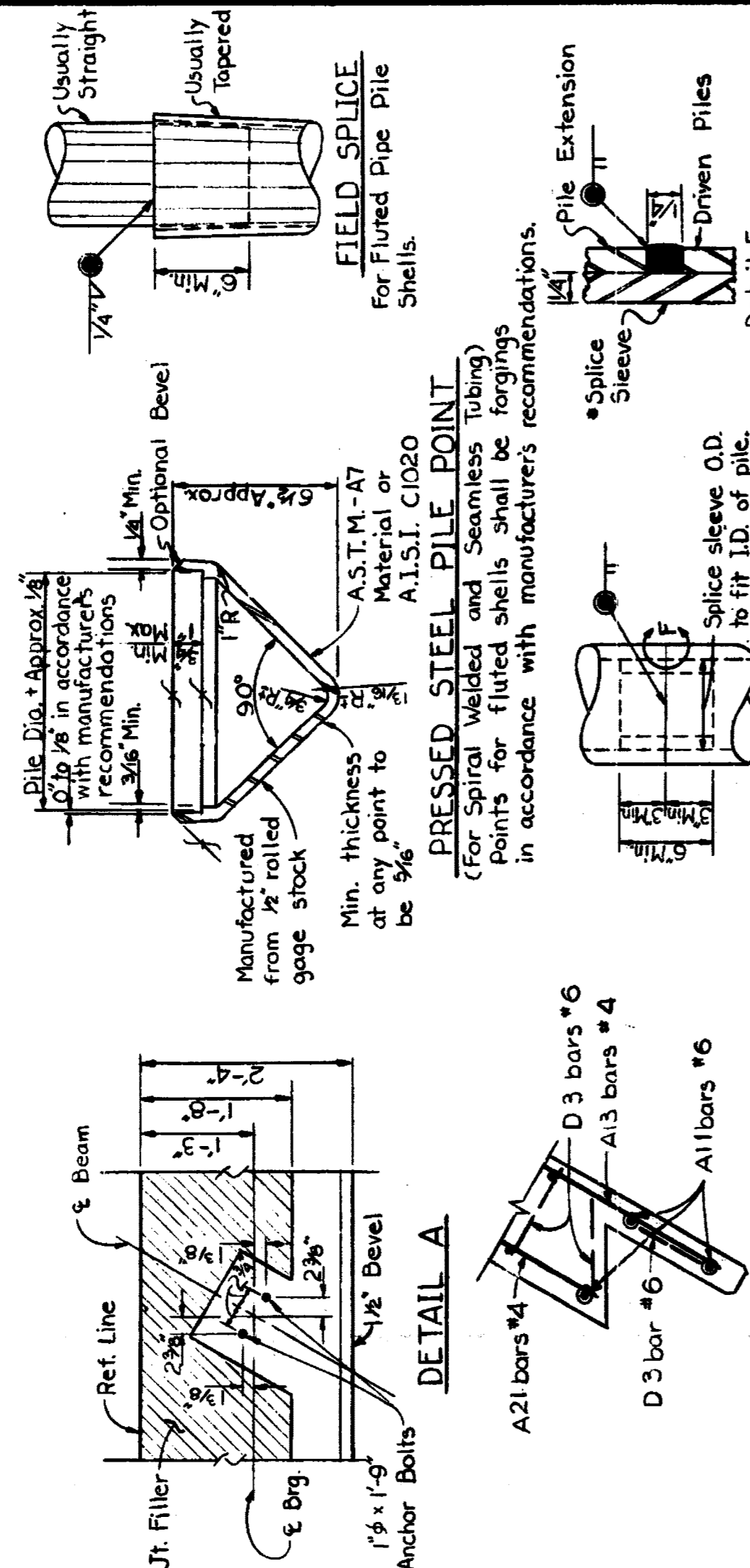
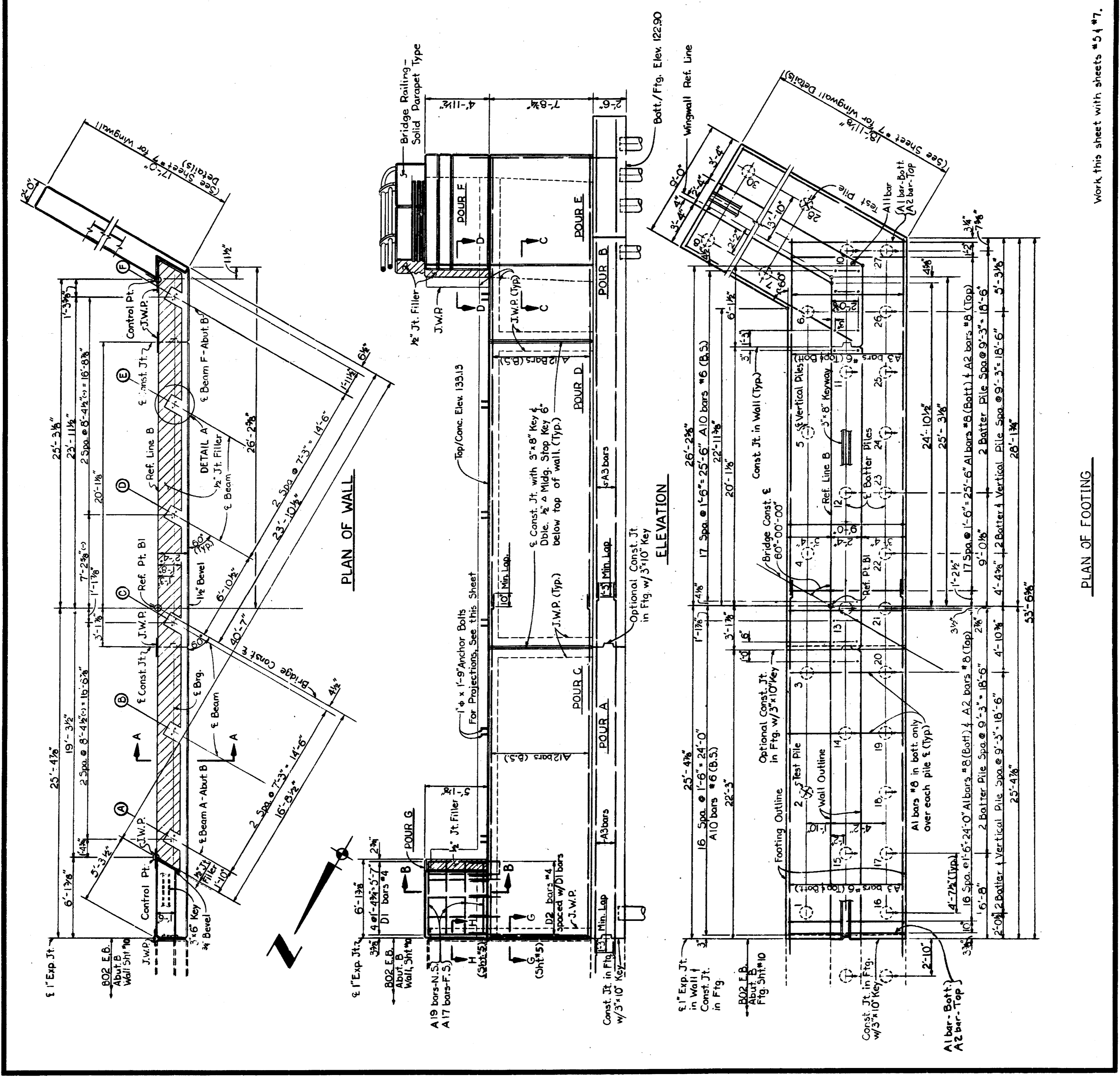
Work this sheet with sheets #61 & #7.

PLAN OF FOOTING

PLAN OF WALL

ELEVATION

Bott. / Elev. 122.70



MISCELLANEOUS QUANTITIES

| ITEM | Unit | Abut. B |
|-----------------------------------|----------|---------|
| Joint Waterproofing | Sq. Ft. | 137 |
| 1/2" Jt. Filler | Sq. Ft. | 90 |
| 1" Jt. Filler | Sq. Ft. | 25 |
| Unclassified Excavation | Cu. Yds. | 330 |
| Bridge Rolling-Solid Parapet Type | Lin. Ft. | 145 |

CONCRETE QUANTITIES

| POUR | ABUTMENT B | (GRADE A/GRADE A/GAA) |
|-----------------------------|------------|-----------------------|
| A | 16.5 | Cu. Yd. |
| B | 37.5 | Cu. Yd. |
| C | 14 | Cu. Yd. |
| D | 15.5 | Cu. Yd. |
| E | 13.8 | Cu. Yd. |
| F | 57 | Cu. Yd. |
| G | 1.6 | Cu. Yd. |
| TOTAL GRADE A(GAA) = | | 56.0 |
| TOTAL GRADE A(GAA) = | | 51.5 |

ANCHOR BOLT PROTECTION

| ABUT. B | PROTECTION |
|---------|------------|
| A | 5 3/4" |
| B | 8 3/4" |
| C | 8 3/4" |
| D | 7 3/4" |
| E | 6 1/4" |
| F | 5 1/2" |

BEAM PROTECTION

| ABUT. B | PROTECTION |
|---------|------------|
| A | 14 |
| B | 15.5 |
| C | 13.8 |
| D | 57 |
| E | 1.6 |
| F | 56.0 |

Work this sheet with sheets #51 #7.

MICHIGAN DEPARTMENT OF STATE HIGHWAYS

ABUTMENT B DETAILS

BOI of 82122J

012500

REVISIONS

| NO. | DATE | BY |
|-----|------|----|
| | | |

ALUMINUM

| SCALE | DATE | BY |
|-------|------|----|
| | | |

REVISIONS

| NO. | DATE | BY |
|-----|------|----|
| | | |

REVISIONS

| NO. | DATE | BY |
|-----|------|----|
| | | |

REVISIONS

| NO. | DATE | BY |
|-----|------|----|
| | | |

REVISIONS

| NO. | DATE | BY |
|-----|------|----|
| | | |

REVISIONS

| NO. | DATE | BY |
|-----|------|----|
| | | |

REVISIONS

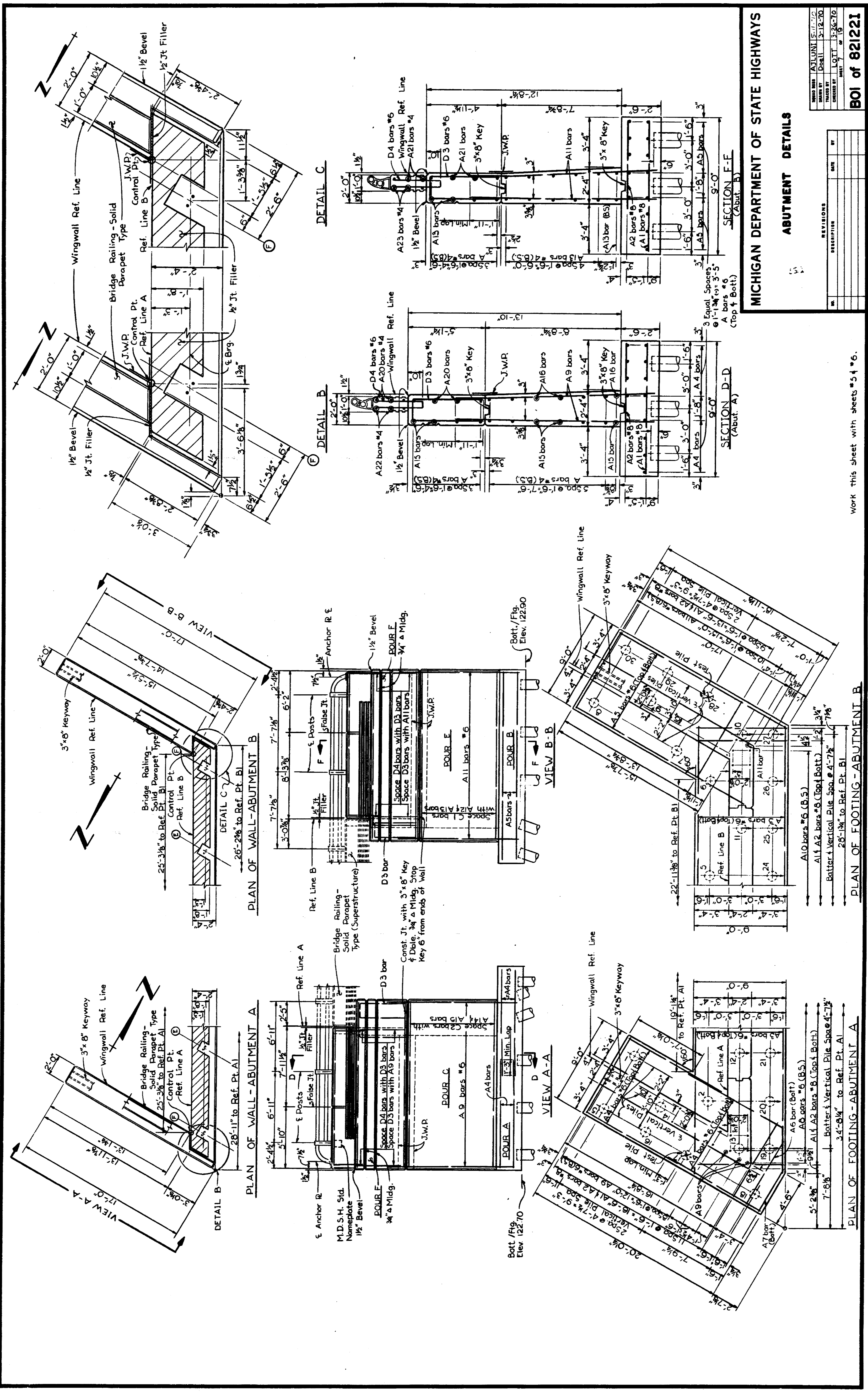
| NO. | DATE | BY |
|-----|------|----|
| | | |

REVISIONS

| NO. | DATE | BY |
|-----|------|----|
| | | |

REVISIONS

| NO. | DATE | BY |
|-----|------|----|
| | | |



MICHIGAN DEPARTMENT OF STATE HIGHWAYS

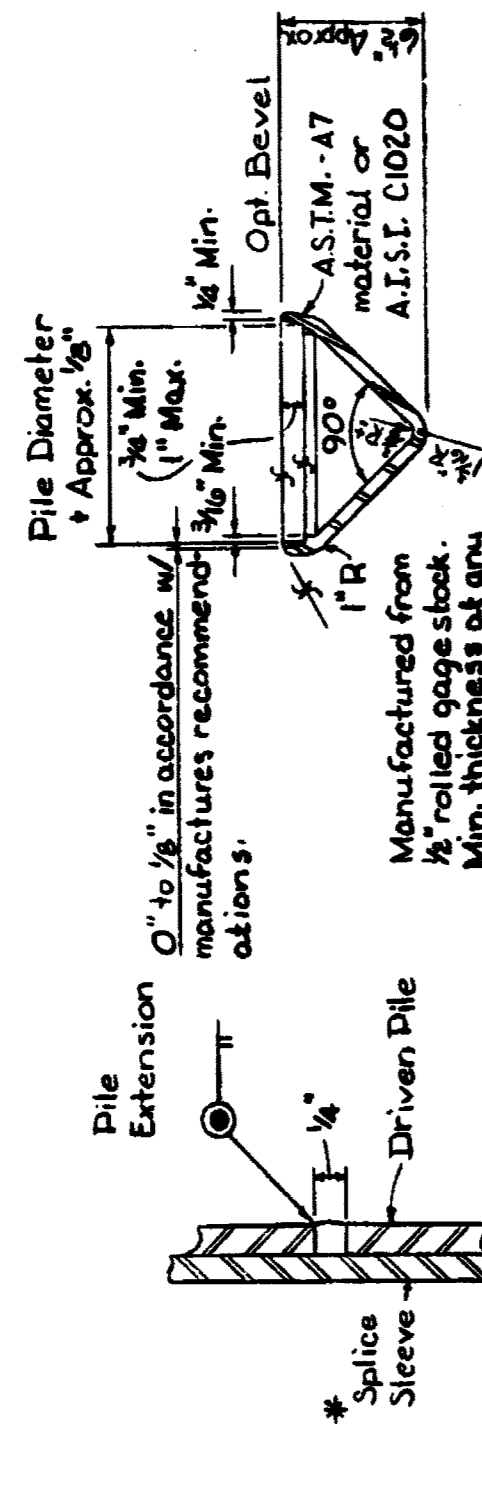
ABUTMENT DETAILS

| | | | |
|-------------|---------|------|---------|
| DESIGNED BY | ATLANTI | DATE | 5-12-70 |
| DRAWN BY | DEAL | DATE | 5-12-70 |
| CHECKED BY | LOTT | DATE | 5-12-70 |
| IN CHARGE | WAT | DATE | 5-12-70 |

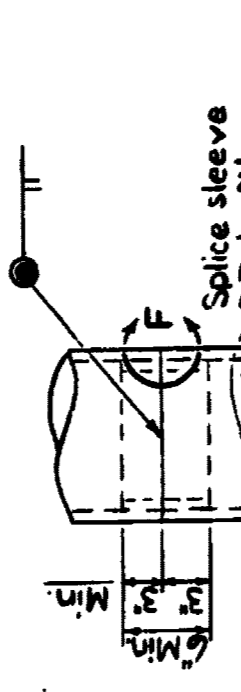
BOI of 81221

012300

Work this sheet with sheets #54 & 6.



PRESSED STEEL PILE POINT
For Spiral Welded and Seamless Tubing.



*** STEEL PIPE SPICE DETAILS**
For Spiral Welded and Seamless Pipe Shells.

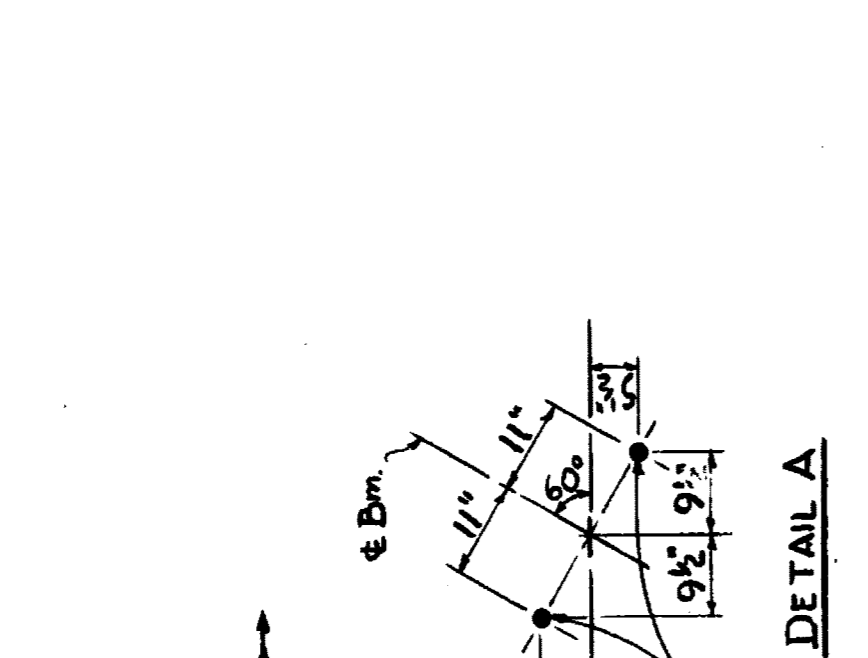
* Split chill rings as recommended by the manufacturer may be substituted splice sleeves at the option of the contractor.

CAST-IN-PLACE CONCRETE PILES

* All piles shall be driven to a minimum bearing capacity of 60 tons. Pile shells for Cast-in-Place Concrete Piles driven without a removable core shall be a minimum of #3 U.S. Standard Gauge (0.250" nominal thickness), 12" O.D. and may be steel pipe of seamless or spiral welded type or fluted pipe as manufactured by the Union Manufacturing Co. or approved equal. Pile points shall be pre-stressed steel or the slip-on type in accordance with the detail shown on the plans. Points shall be attached to pile shells with 1/2" continuous weld.

| CAST-IN-PLACE CONCRETE PILES | | | |
|------------------------------|--------------|--------------|-------------------------|
| Location | Type of Pile | No. of Piles | Total Est. Length (ea.) |
| Pier 1 | Vert. | 17 | 18' |
| | Test | 1 | 28' |
| Pier 2 | Vert. | 17 | 18' |
| | Test | 1 | 28' |
| Total | | | 668' |

| MISCELLANEOUS QUANTITIES | | | |
|--------------------------|----------|------------|------------|
| Item | Unit | Pier 1 | Pier 2 |
| Unclassified Excavation | Cu. Yds. | 206 | 206 |
| Total | | 412 | 412 |



FIELD SPICE
For Fluted Pipe Pile Shells.

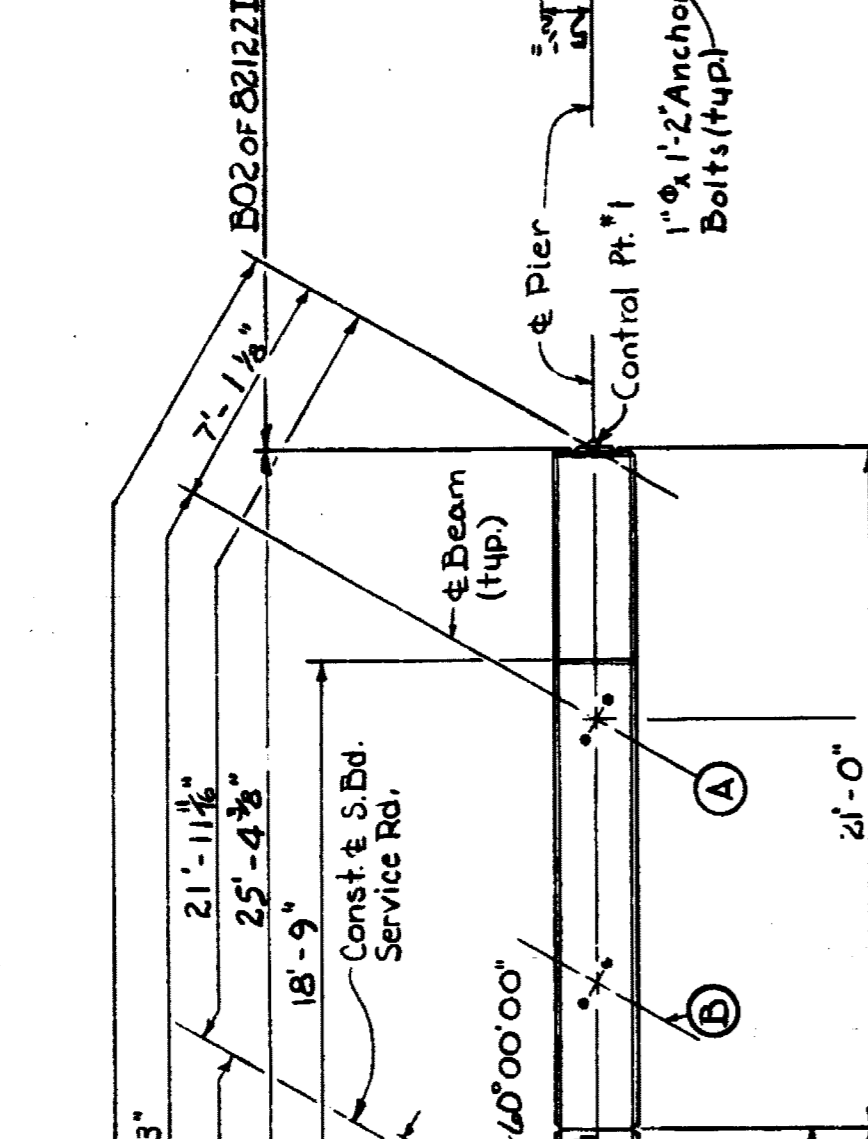
* Split chill rings as recommended by the manufacturer may be substituted splice sleeves at the option of the contractor.

CAST-IN-PLACE CONCRETE PILES

* All piles shall be driven to a minimum bearing capacity of 60 tons. Pile shells for Cast-in-Place Concrete Piles driven without a removable core shall be a minimum of #3 U.S. Standard Gauge (0.250" nominal thickness), 12" O.D. and may be steel pipe of seamless or spiral welded type or fluted pipe as manufactured by the Union Manufacturing Co. or approved equal. Pile points shall be pre-stressed steel or the slip-on type in accordance with the detail shown on the plans. Points shall be attached to pile shells with 1/2" continuous weld.

| CAST-IN-PLACE CONCRETE PILES | | | |
|------------------------------|--------------|--------------|-------------------------|
| Location | Type of Pile | No. of Piles | Total Est. Length (ea.) |
| Pier 1 | Vert. | 17 | 18' |
| | Test | 1 | 28' |
| Pier 2 | Vert. | 17 | 18' |
| | Test | 1 | 28' |
| Total | | | 668' |

| MISCELLANEOUS QUANTITIES | | | |
|--------------------------|----------|------------|------------|
| Item | Unit | Pier 1 | Pier 2 |
| Unclassified Excavation | Cu. Yds. | 206 | 206 |
| Total | | 412 | 412 |



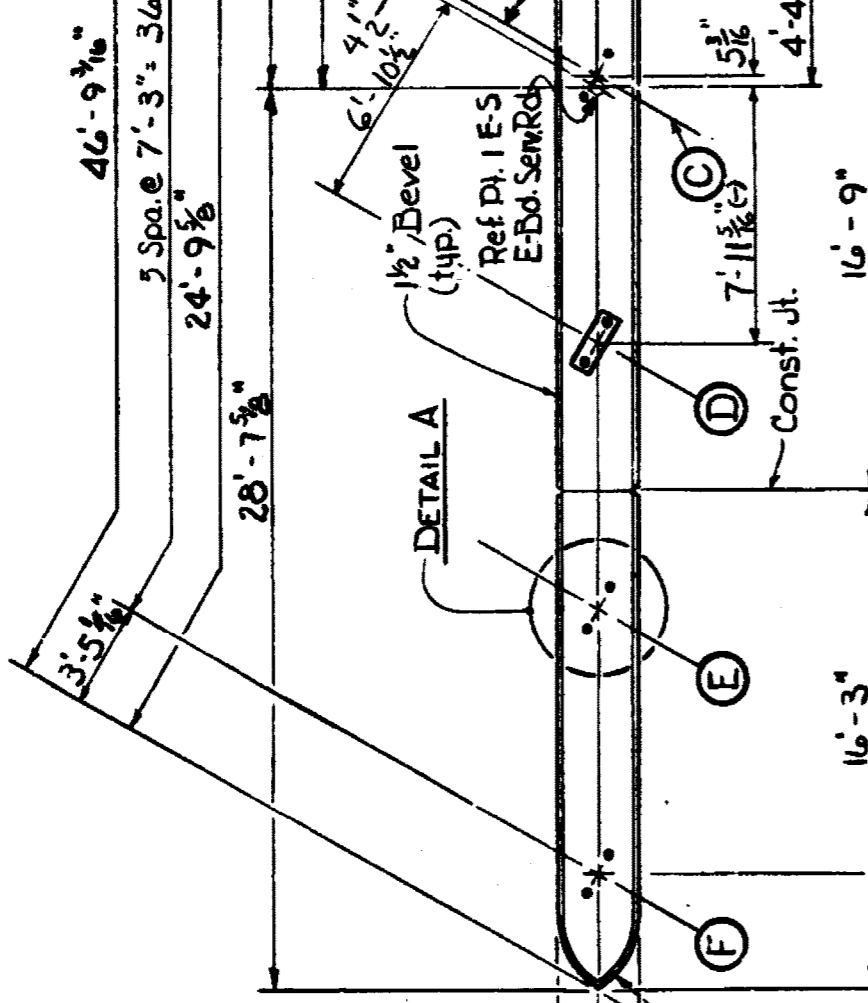
PIER I DETAILS

* Split chill rings as recommended by the manufacturer may be substituted splice sleeves at the option of the contractor.

CAST-IN-PLACE CONCRETE PILES

| CAST-IN-PLACE CONCRETE PILES | | | |
|------------------------------|--------------|--------------|-------------------------|
| Location | Type of Pile | No. of Piles | Total Est. Length (ea.) |
| Pier 1 | Vert. | 17 | 18' |
| | Test | 1 | 28' |
| Pier 2 | Vert. | 17 | 18' |
| | Test | 1 | 28' |
| Total | | | 668' |

| MISCELLANEOUS QUANTITIES | | | |
|--------------------------|----------|------------|------------|
| Item | Unit | Pier 1 | Pier 2 |
| Unclassified Excavation | Cu. Yds. | 206 | 206 |
| Total | | 412 | 412 |



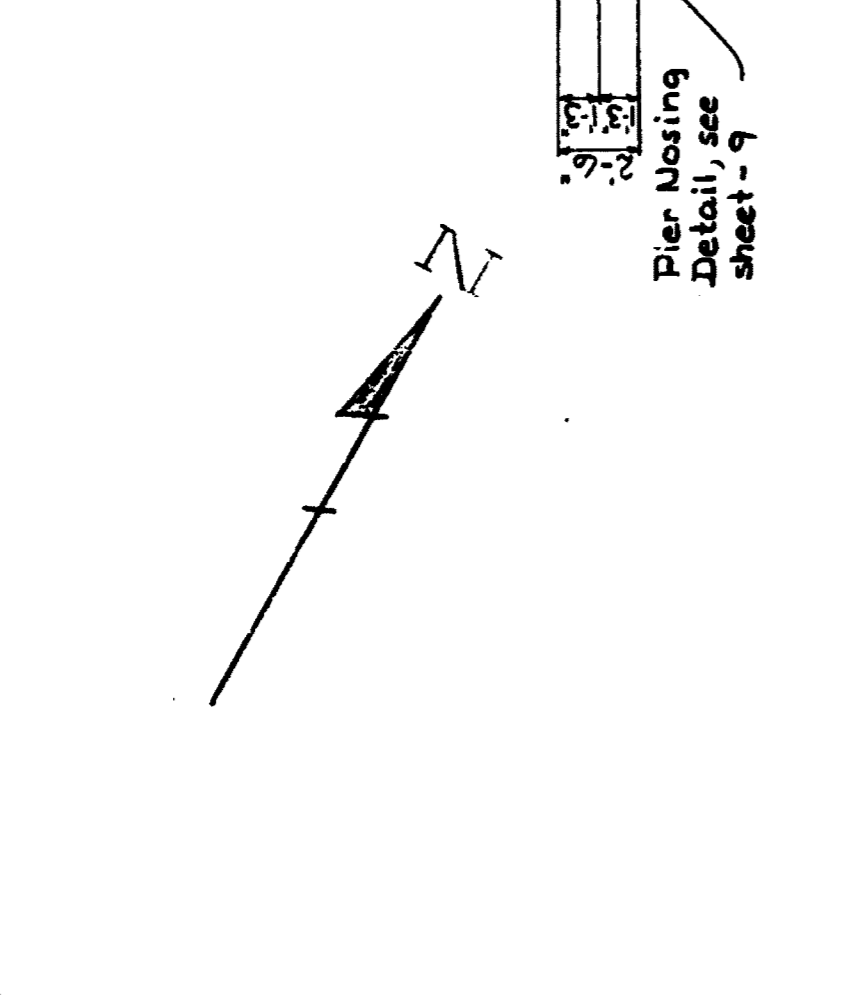
SECTION A-A

* Split chill rings as recommended by the manufacturer may be substituted splice sleeves at the option of the contractor.

CAST-IN-PLACE CONCRETE PILES

| CAST-IN-PLACE CONCRETE PILES | | | |
|------------------------------|--------------|--------------|-------------------------|
| Location | Type of Pile | No. of Piles | Total Est. Length (ea.) |
| Pier 1 | Vert. | 17 | 18' |
| | Test | 1 | 28' |
| Pier 2 | Vert. | 17 | 18' |
| | Test | 1 | 28' |
| Total | | | 668' |

| MISCELLANEOUS QUANTITIES | | | |
|--------------------------|----------|------------|------------|
| Item | Unit | Pier 1 | Pier 2 |
| Unclassified Excavation | Cu. Yds. | 206 | 206 |
| Total | | 412 | 412 |



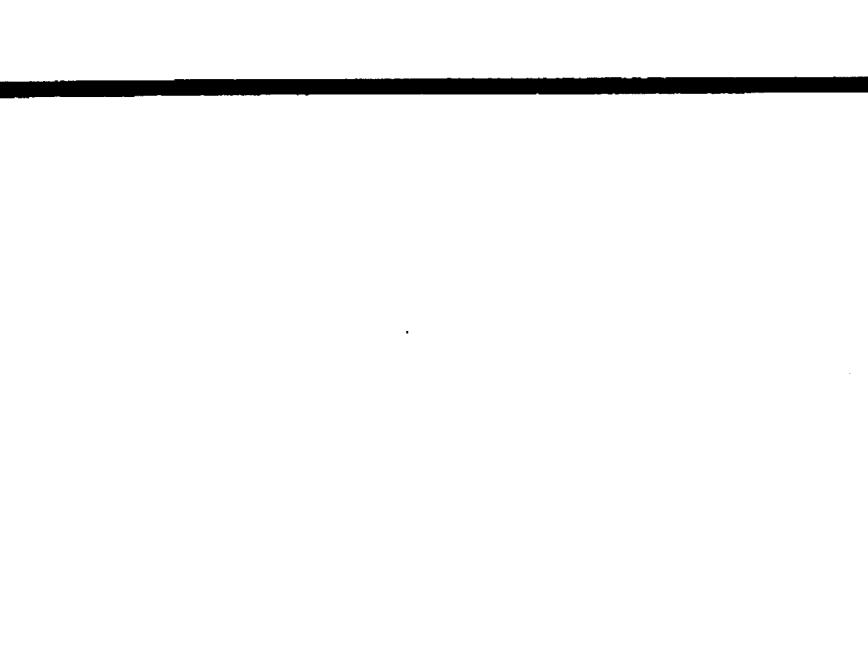
DETAIL B

* Split chill rings as recommended by the manufacturer may be substituted splice sleeves at the option of the contractor.

CAST-IN-PLACE CONCRETE PILES

| CAST-IN-PLACE CONCRETE PILES | | | |
|------------------------------|--------------|--------------|-------------------------|
| Location | Type of Pile | No. of Piles | Total Est. Length (ea.) |
| Pier 1 | Vert. | 17 | 18' |
| | Test | 1 | 28' |
| Pier 2 | Vert. | 17 | 18' |
| | Test | 1 | 28' |
| Total | | | 668' |

| MISCELLANEOUS QUANTITIES | | | |
|--------------------------|----------|------------|------------|
| Item | Unit | Pier 1 | Pier 2 |
| Unclassified Excavation | Cu. Yds. | 206 | 206 |
| Total | | 412 | 412 |



PLAN OF FOOTING

* Split chill rings as recommended by the manufacturer may be substituted splice sleeves at the option of the contractor.

CAST-IN-PLACE CONCRETE PILES

| CAST-IN-PLACE CONCRETE PILES | | | |
|------------------------------|--------------|--------------|-------------------------|
| Location | Type of Pile | No. of Piles | Total Est. Length (ea.) |
| Pier 1 | Vert. | 17 | 18' |
| | Test | 1 | 28' |
| Pier 2 | Vert. | 17 | 18' |
| | Test | 1 | 28' |
| Total | | | 668' |

| MISCELLANEOUS QUANTITIES | | | |
|--------------------------|----------|------------|------------|
| Item | Unit | Pier 1 | Pier 2 |
| Unclassified Excavation | Cu. Yds. | 206 | 206 |
| Total | | 412 | 412 |

MICHIGAN DEPARTMENT OF STATE HIGHWAYS

PIER I DETAILS

Work this sheet with sheet 9.

BOI of 821221

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REVISIONS

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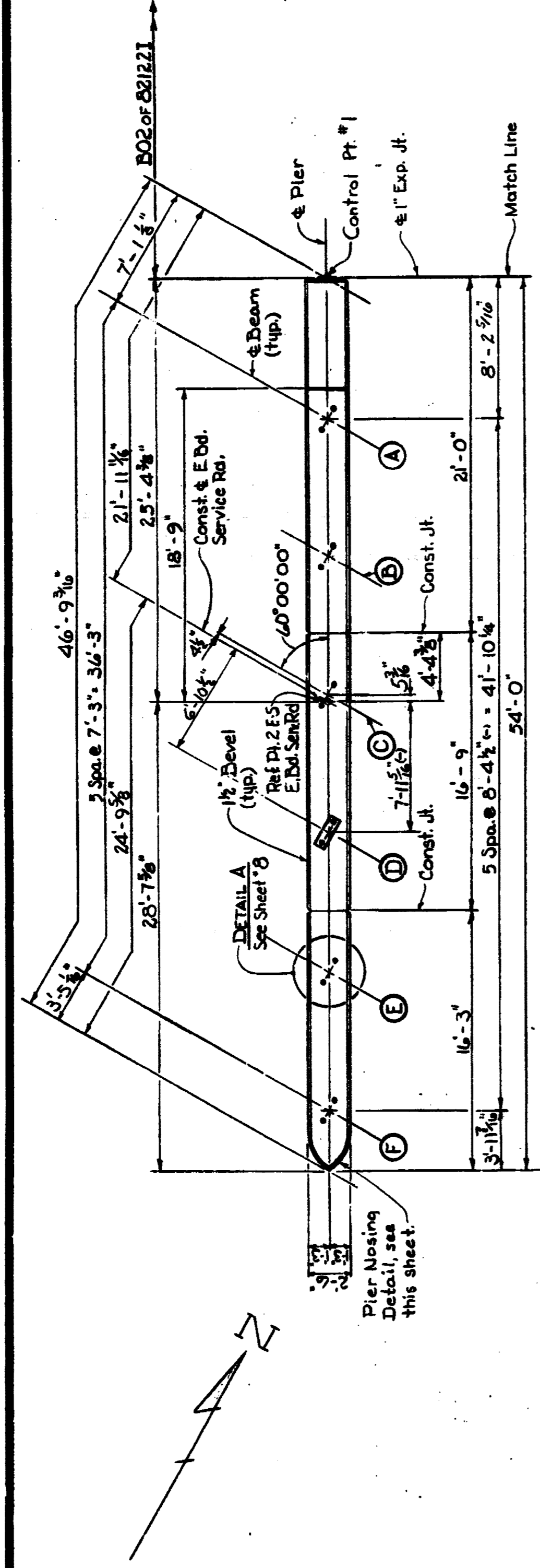
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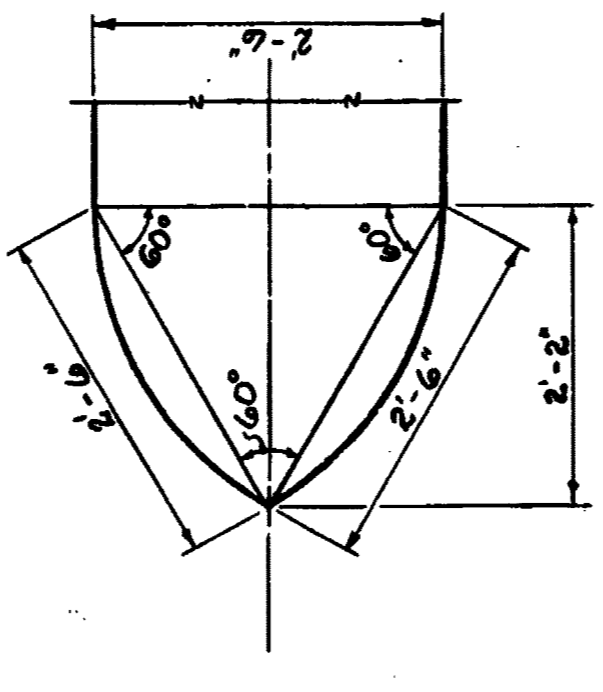
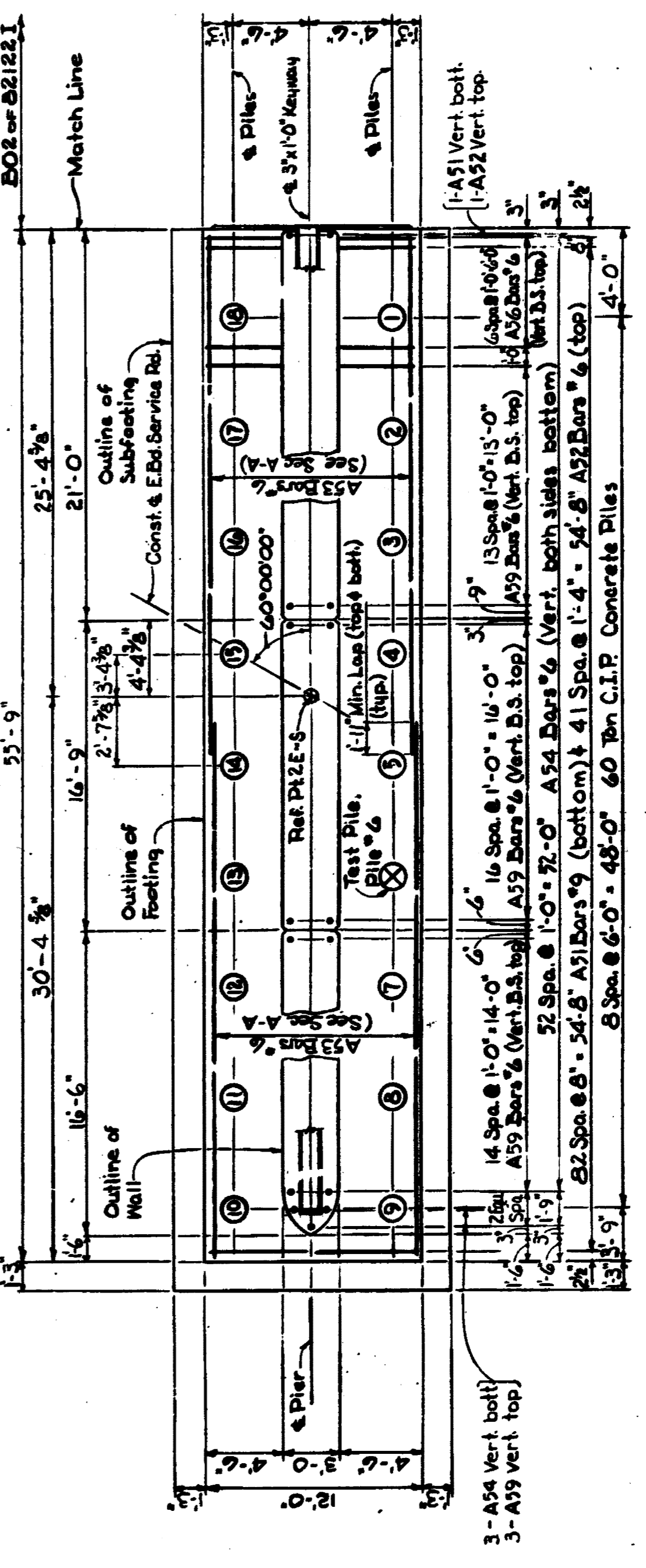
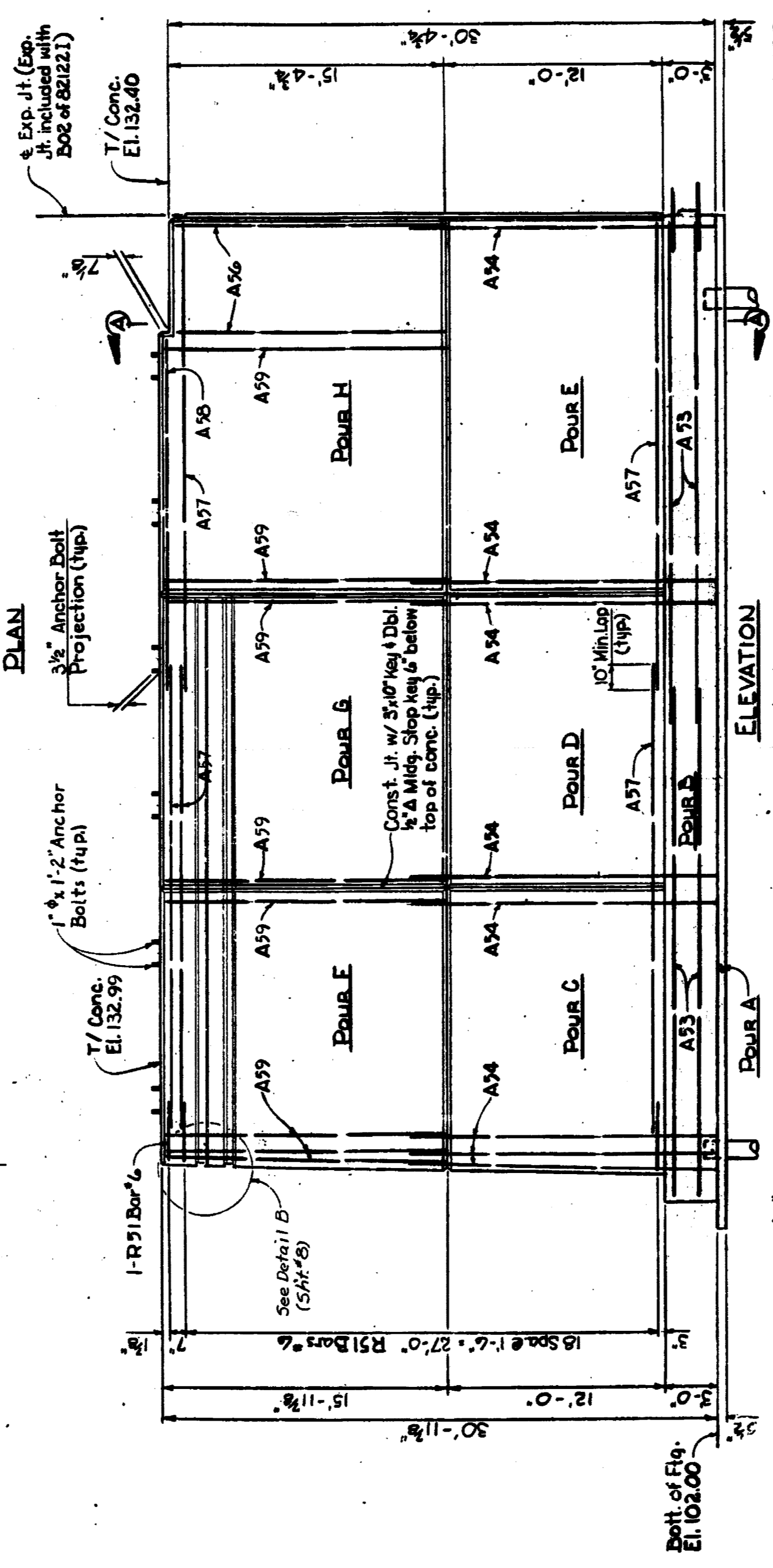
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DATE: 10-24-59

SHEET 8 OF 19



| POUR | CONCRETE QUANTITIES CU. YDS. | |
|---|------------------------------|----------------|
| | PIER 1 | PIER 2 |
| A | 74.3 | 74.3 |
| B | 19.3 | 18.8 |
| C | 20.9 | 21.0 |
| D | 26.2 | 26.3 |
| E | 24.1 | 23.7 |
| F | 26.5 | 26.2 |
| G | 32.8 | 32.5 |
| H | 149.8 | 148.3 |
| Sub-Total | | 74.3 |
| Total Grade A (GAA) Concrete | | 148.6 Cu. Yds. |
| Total Grade A (GAA) Concrete | | 298.1 Cu. Yds. |
| * A - Concrete Subfootings tot 2 Piers 280 Cu. Yds. | | |



PIER NOSING DETAIL

Notes:
 B.S. denotes Both Sides.
 For bevel and molding details, see standard sheet R16.
 Anchor bolts shall be accurately set to a template.
 For pile quantities, see sheet 8.

Work this sheet with sheet # 8.

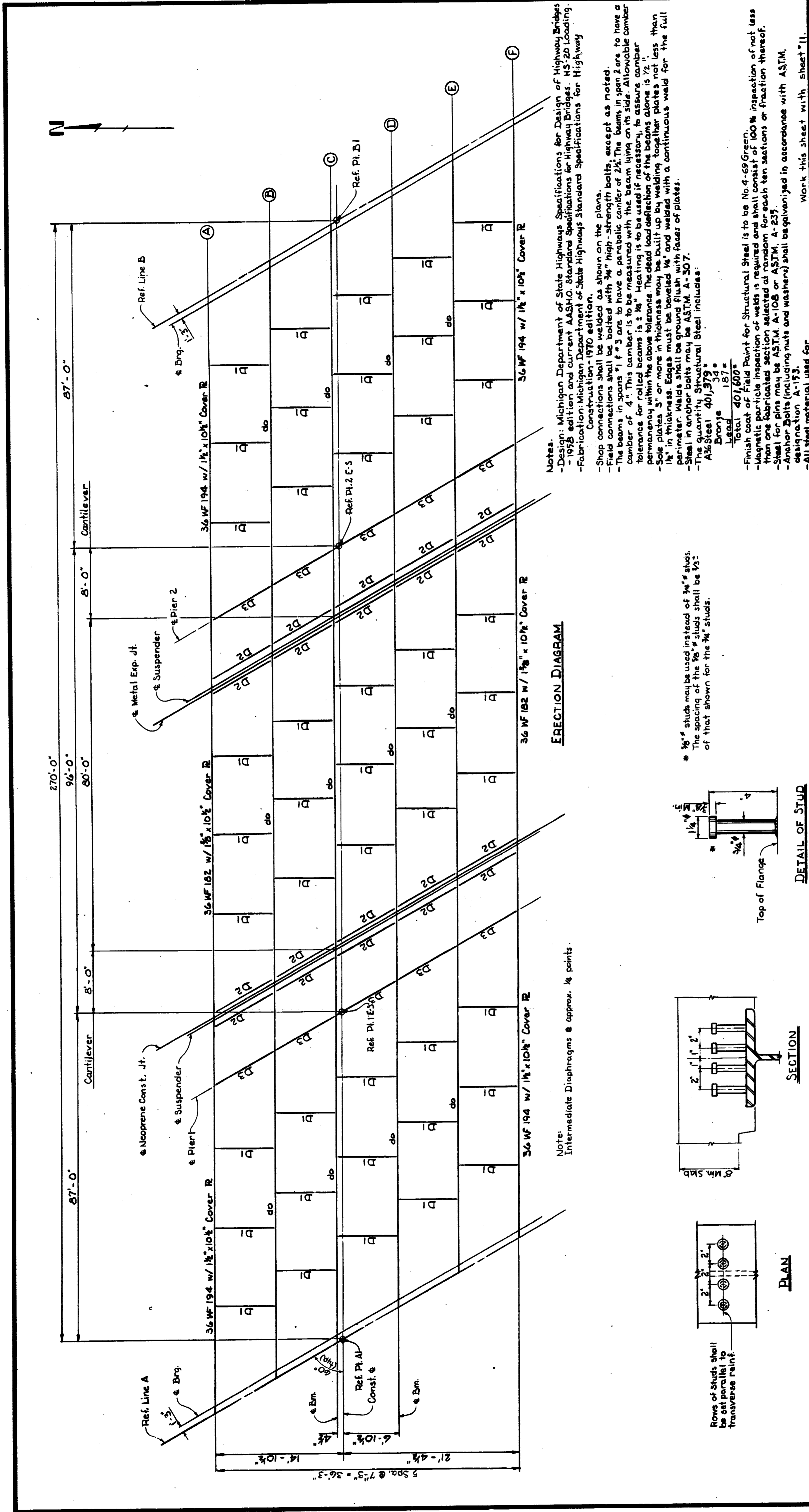
MICHIGAN DEPARTMENT OF STATE HIGHWAYS

PIER 2 DETAILS

BOI of 821221

| NO. | REVISIONS | DATE | BY |
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| 1 | | | |
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DATE: JULY 15, 1972
 DRAWN BY: GILLER
 CHECKED BY: R. B. BARNARD
 BOI: 821221



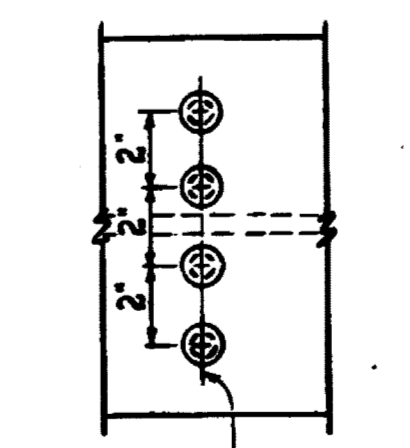
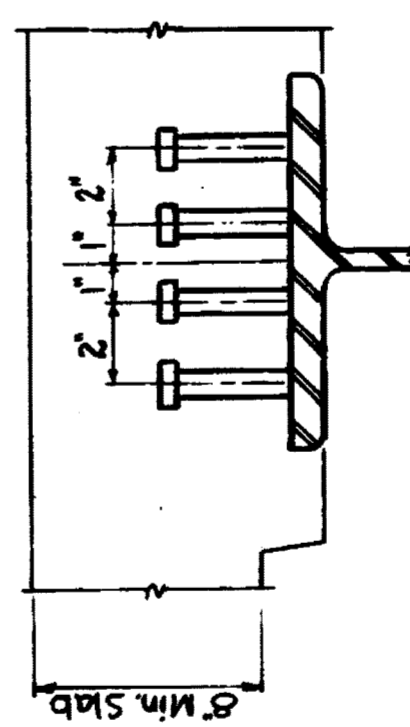
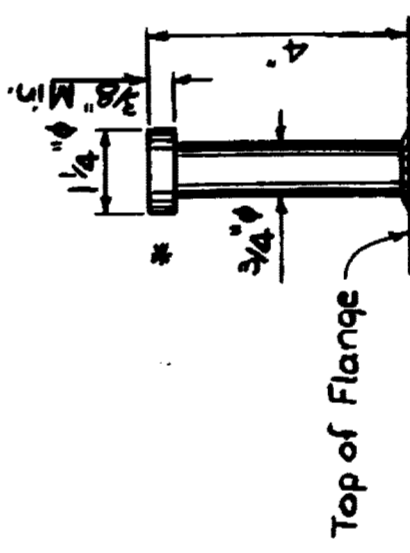
Notes:

- Design: Michigan Department of State Highways Specifications for Design of Highway Bridges - 1975 edition and current AASHTO Standard Specifications for Highway Bridges, 15-20 Loading.
- Fabrication: Michigan Department of State Highways Standard Specifications for Highway Construction - 1970 edition.
- Shop connections shall be welded as shown on the plans.
- Field connections shall be bolted with 3/4" high-strength bolts, except as noted.
- The beams in spans #1 & #3 are to have a parabolic camber of 2". The beams in span #2 are to have a camber of 4". This camber is to be measured with the beam lying on its side. Allowable camber tolerance for rolled beams is $\pm 1/8"$. Heating is to be used if necessary, to assure camber permanence within the above tolerance. The dead load deflection of the beams alone is $1/2"$.
- Sole plates 3" or more in thickness may be built up by welding together plates not less than 1/2" in thickness. Edges must be beveled 1/4" and welded with a continuous weld for the full perimeter. Welds shall be ground flush with faces of plates.
- Steel in anchor bolts may be ASTM A-307.
- The quantity Structural Steel includes:
 - A36 Steel 401,379#
 - Brnzs 187#
 - Total 401,600#
- Finish coat of Field Paint for Structural Steel is to be No. 4-69 Green.
- Magnetic particle inspection of welds is required and shall consist of 100% inspection of not less than one fabricated section selected at random for each ten sections or fraction thereof.
- Steel for pins may be ASTM A-108 or ASTM A-237.
- Anchor Bolts (including nuts and washers) shall be galvanized in accordance with ASTM designation A-153.
- All steel material used for bearings, with exception of portion welded to beams, shall be galvanized in accordance with ASTM Designation A-153. Galvanizing shall be applied after fabrication of bearing, mill scale oxidation material shall be removed prior to galvanizing. Steel to be galvanized shall be ASTM A-307, ASTM B-22.

ERECTION DIAGRAM

Note: Intermediate Diaphragms @ approx. 1/4 points.

* 3/8" studs may be used instead of 1/2" studs. The spacing of the 3/8" studs shall be 1/2' of that shown for the 1/2" studs.



Rows of Studs shall be set parallel to transverse reinforcement.

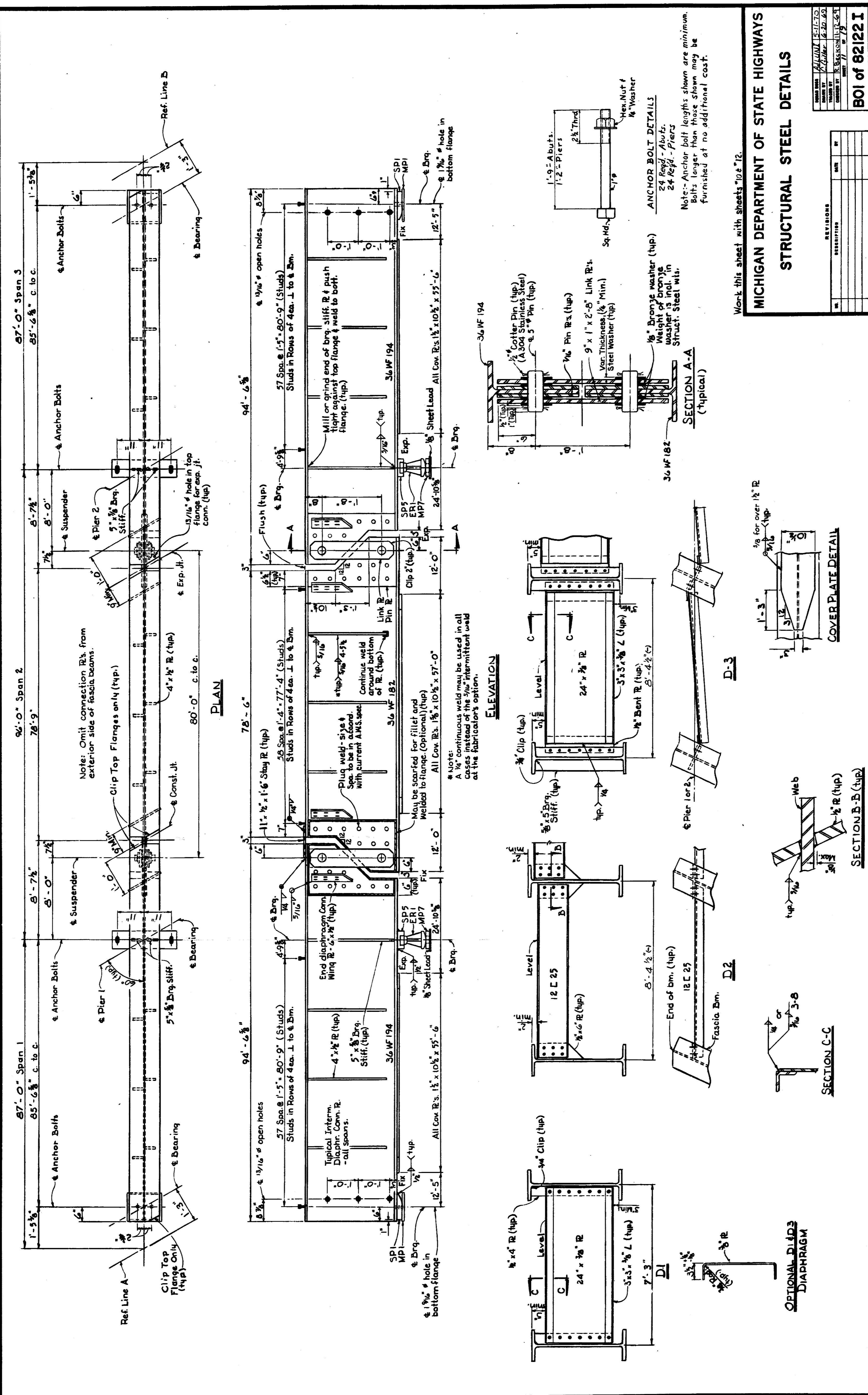
| STRUCTURAL STEEL QUANTITIES | | |
|--------------------------------|----------|---------|
| Item | Unit | Amount |
| Structural Steel - Fabricating | Lbs. | 401,600 |
| Structural Steel - Erection | Lbs. | 401,600 |
| Shear Developers | Lump Sum | |
| Field Painting | Lump Sum | |

STUD SHEAR DEVELOPER DETAILS

Notes: Welding of Studs to beam flanges is incidental to Shear Developers. Weight of Studs is not included in Structural Steel Weights.

MICHIGAN DEPARTMENT OF STATE HIGHWAYS
STRUCTURAL STEEL DETAILS

| | |
|-------------|-------------|
| DESIGNED BY | DATE |
| CHECKED BY | DATE |
| APPROVED BY | DATE |
| REVISIONS | |
| NO. | DESCRIPTION |
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| | |



94'-0" Span 1
85'-6 3/8" c. to c.

94'-0" Span 2
85'-6 3/8" c. to c.

94'-0" Span 3
85'-6 3/8" c. to c.

PLAN

ELEVATION

Work this sheet with sheets 702 & 72.

**MICHIGAN DEPARTMENT OF STATE HIGHWAYS
STRUCTURAL STEEL DETAILS**

| NO. | REVISIONS | DATE | BY |
|-----|-----------|------|----|
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Note: Omit connection R's from exterior side of fascia beams.

Note: A 1/2" continuous weld may be used in all cases instead of the 5/16" intermittent weld at the fabricator's option.

Note: Anchor bolt lengths shown are minimum. Bolts longer than those shown may be furnished at no additional cost.

OPTIONAL D1/D3 DIAPHRAGM

SECTION C-C

SECTION B-B (typ)

COVER PLATE DETAIL

D2

D3

SECTION A-A (typical)

ANCHOR BOLT DETAILS

24 Reqd - Abuts.
24 Reqd - Piers

| BEAM | TYPE | L | W | t | P | k | S |
|----------|------|-----------|--------|---|---|--------|--------|
| A | SPI | 1'-0" | 2" | | | | 0 |
| B | do | do | 3" | | | | do |
| C | do | do | 3 1/2" | | | | do |
| D | do | do | 3 3/4" | | | | do |
| E | do | do | 3 1/2" | | | | do |
| F | do | do | 3 3/4" | | | | do |
| A thru F | SPI | 1'-0" | 3 3/4" | | | | 0 |
| | MPI | 1'-1" | | | | | |
| A | SP5 | 1'-0 1/2" | 3" | | | | 0 |
| B | do | do | 3 3/4" | | | | do |
| C | do | do | 4" | | | | do |
| D | do | do | 4 1/2" | | | | do |
| E | do | do | 4 3/4" | | | | do |
| F | do | do | 4 1/2" | | | | do |
| A thru F | SP5 | 1'-0 3/4" | 4 1/4" | | | | 0 |
| A | SP5 | 1'-0 3/4" | 5 1/4" | | | | -1/8" |
| B | do | do | 5 1/2" | | | | do |
| C | do | do | 5 3/4" | | | | do |
| D | do | do | 5 1/2" | | | | do |
| E | do | do | 5 3/4" | | | | do |
| F | do | do | 5 1/2" | | | | do |
| A thru F | SP5 | 1'-0 3/4" | 5 3/4" | | | | -1/8" |
| A | MP7 | 2'-2" | | | | 5 1/2" | 1 1/2" |
| B | do | do | | | | do | do |
| C | do | do | | | | do | do |
| D | do | do | | | | do | do |
| E | do | do | | | | do | do |
| F | do | do | | | | do | do |
| A thru F | MP7 | 2'-2" | | | | 5 1/2" | 1 1/2" |
| A | SPI | 1'-0" | 5 1/4" | | | | +1/8" |
| B | do | do | 5 1/2" | | | | do |
| C | do | do | 5 3/4" | | | | do |
| D | do | do | 5 1/2" | | | | do |
| E | do | do | 5 3/4" | | | | do |
| F | do | do | 5 1/2" | | | | do |
| A thru F | SPI | 1'-0" | 5 3/4" | | | | +1/8" |
| | MPI | 1'-1" | | | | | |
| A thru F | SPI | 1'-0" | 2" | | | | 0 |
| | MPI | 1'-1" | | | | | |

MICHIGAN STATE HIGHWAY DEPARTMENT
BEARING DETAILS

REVISIONS

DATE

BY

NO.

DESCRIPTION

DATE

BY

NO.

DESCRIPTION

DATE

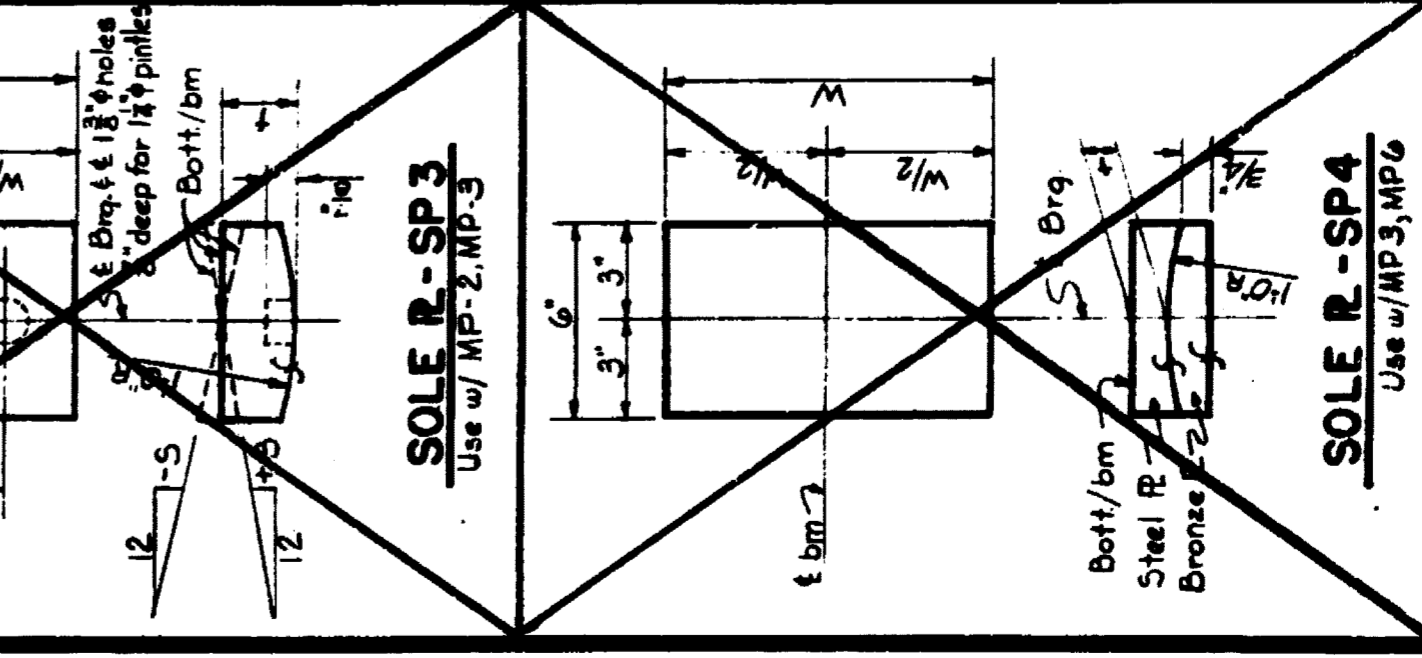
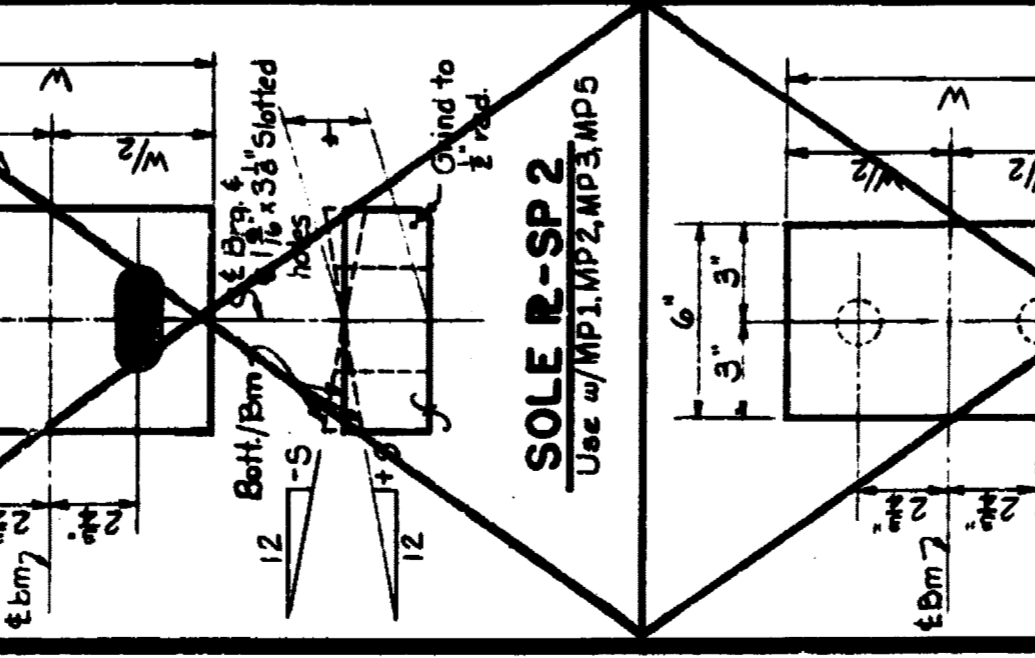
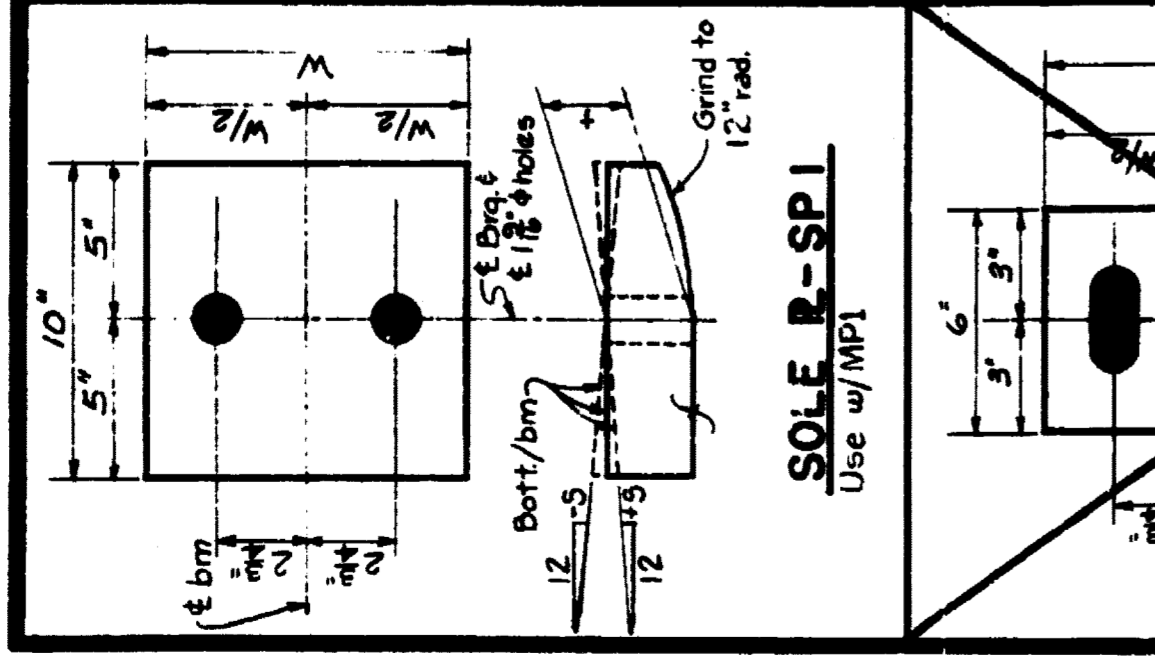
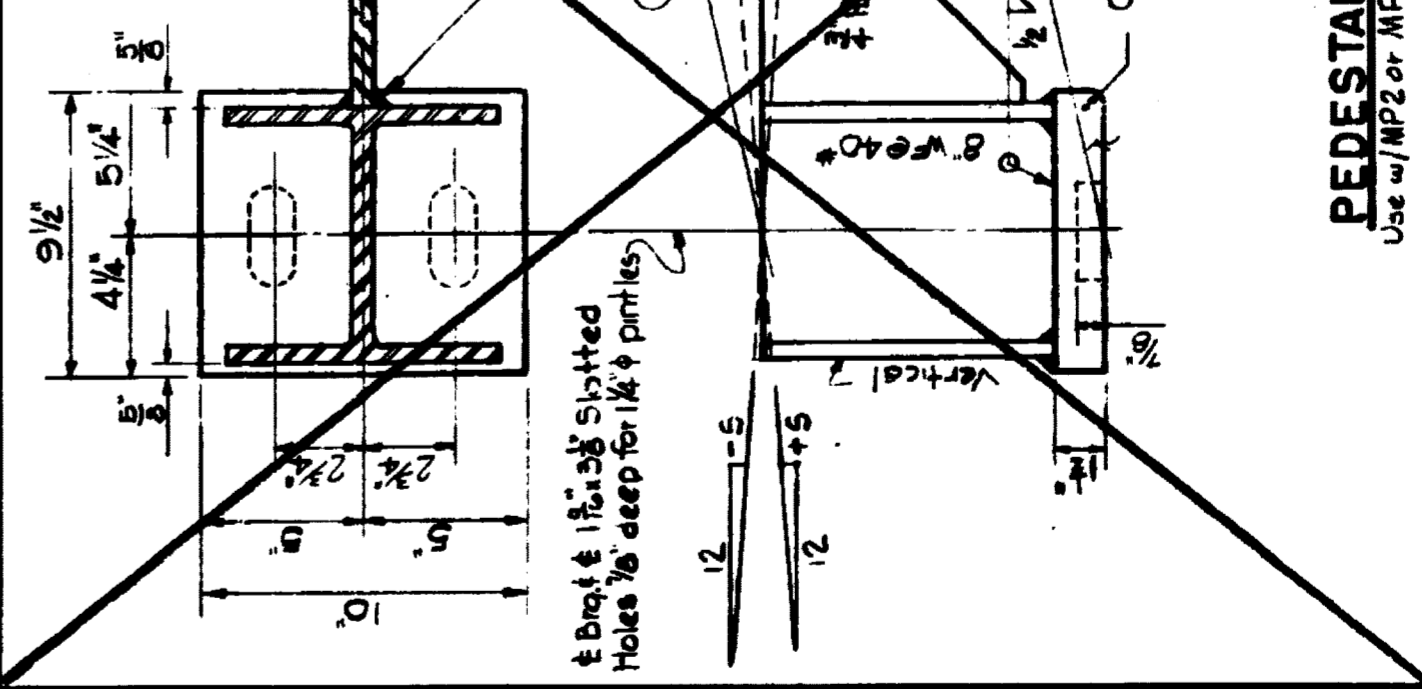
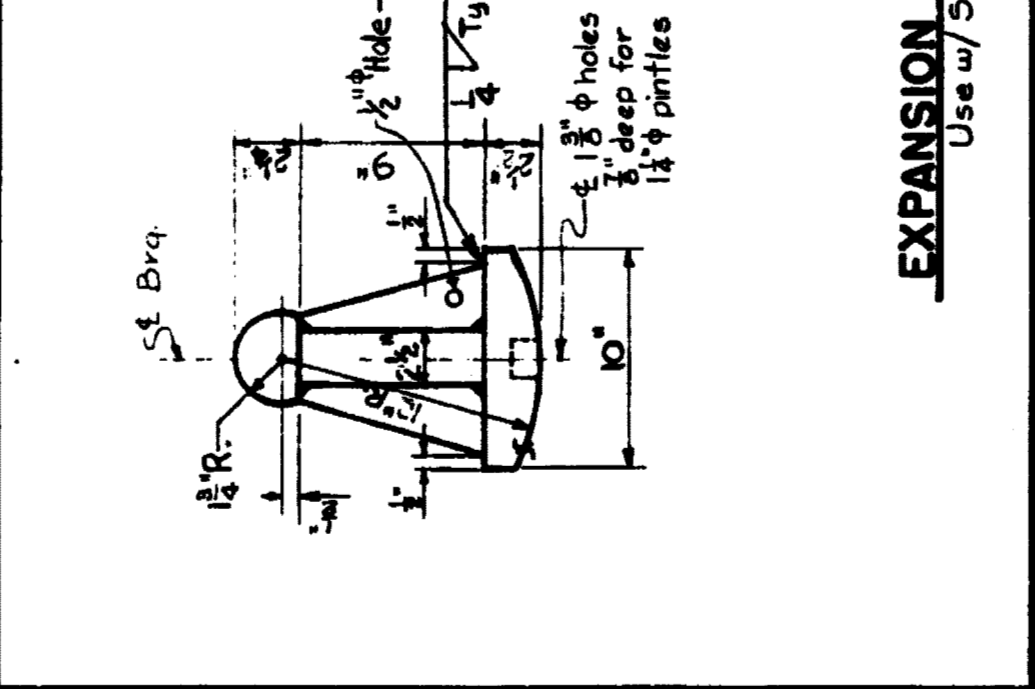
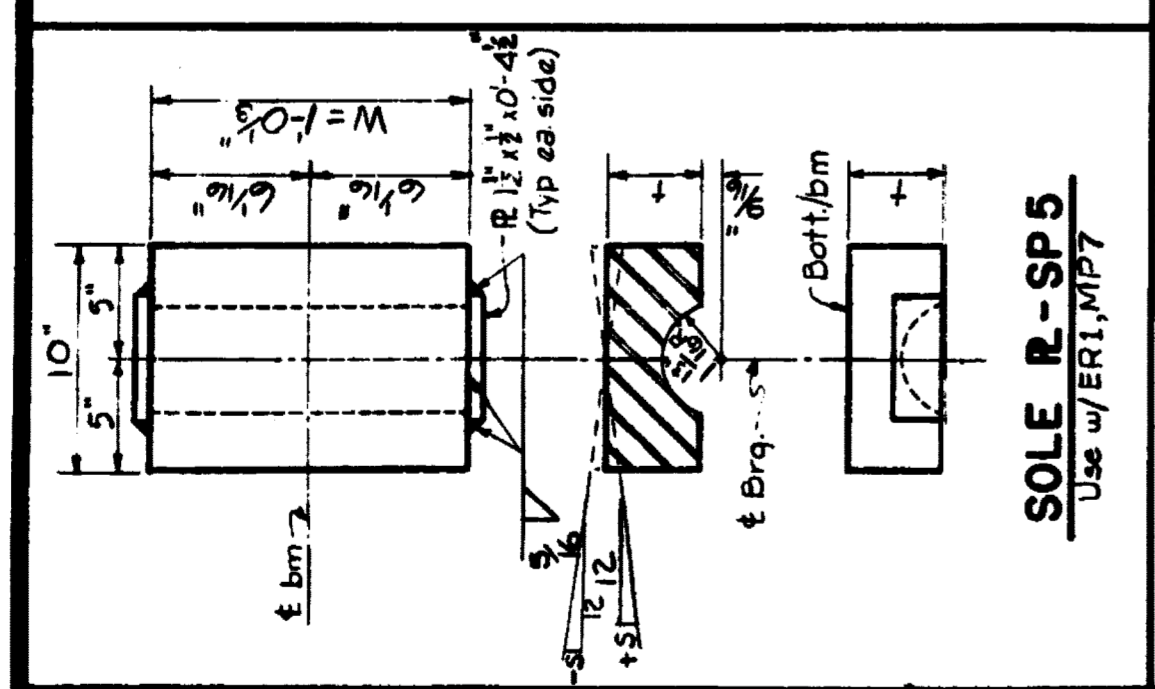
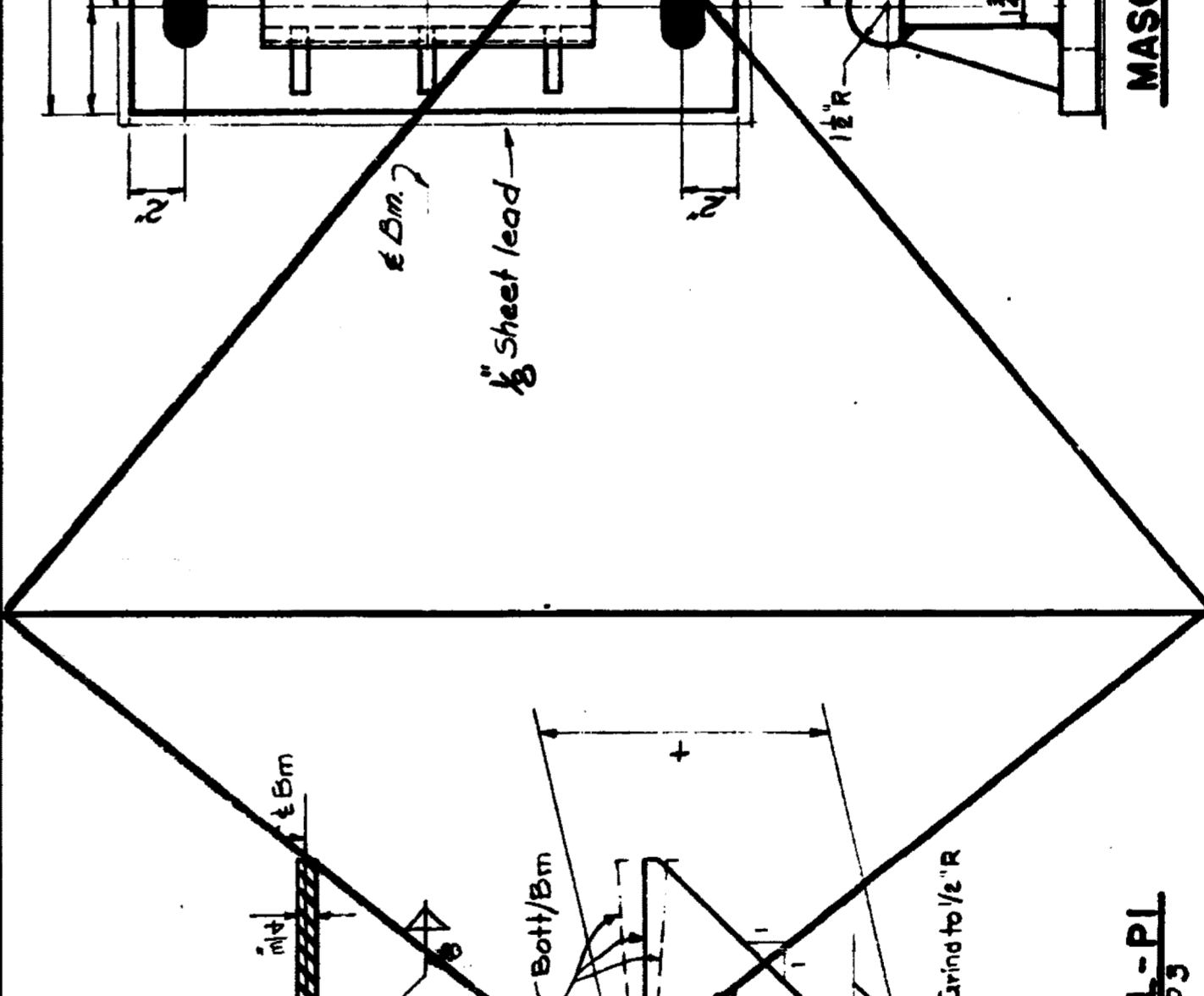
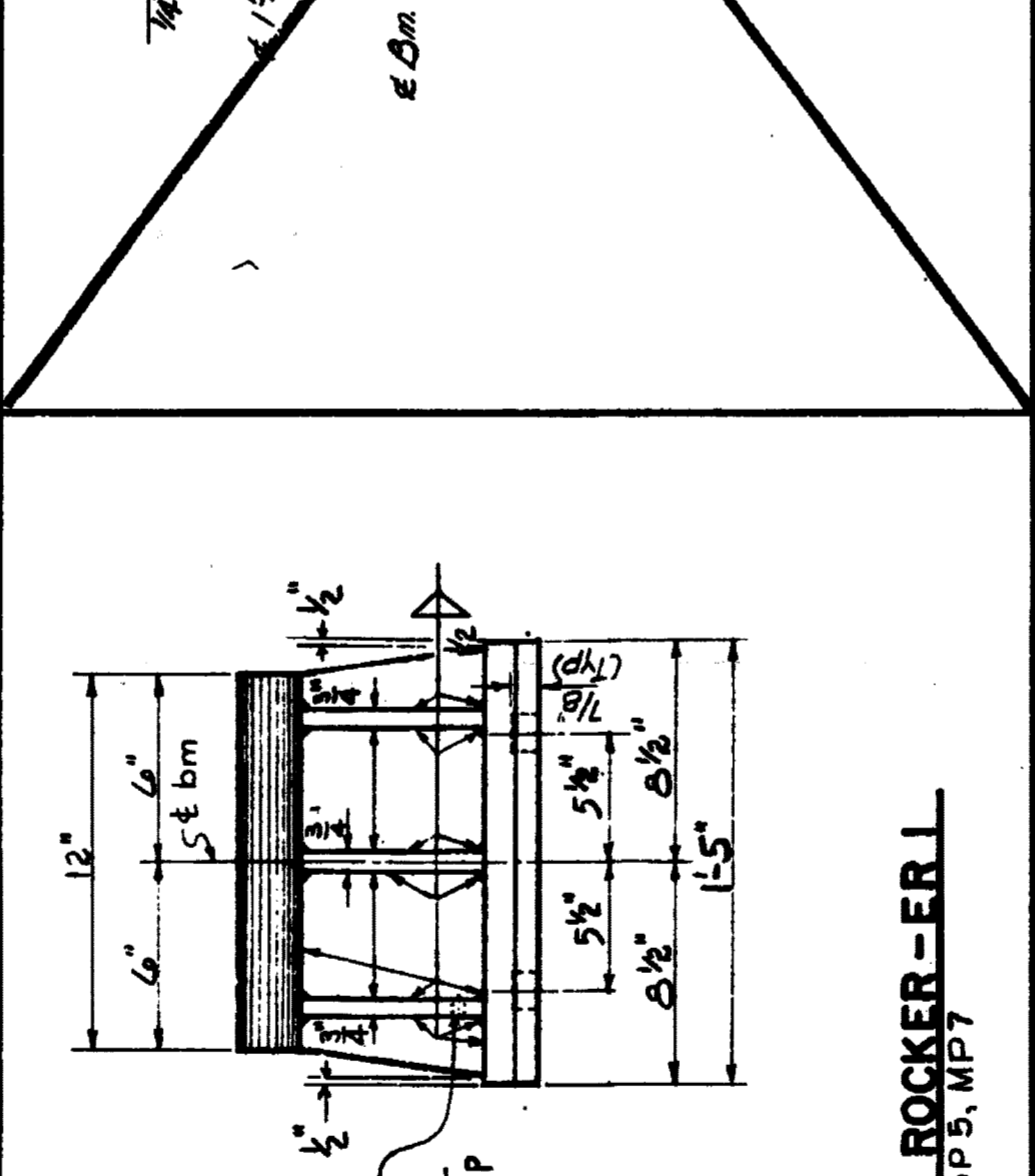
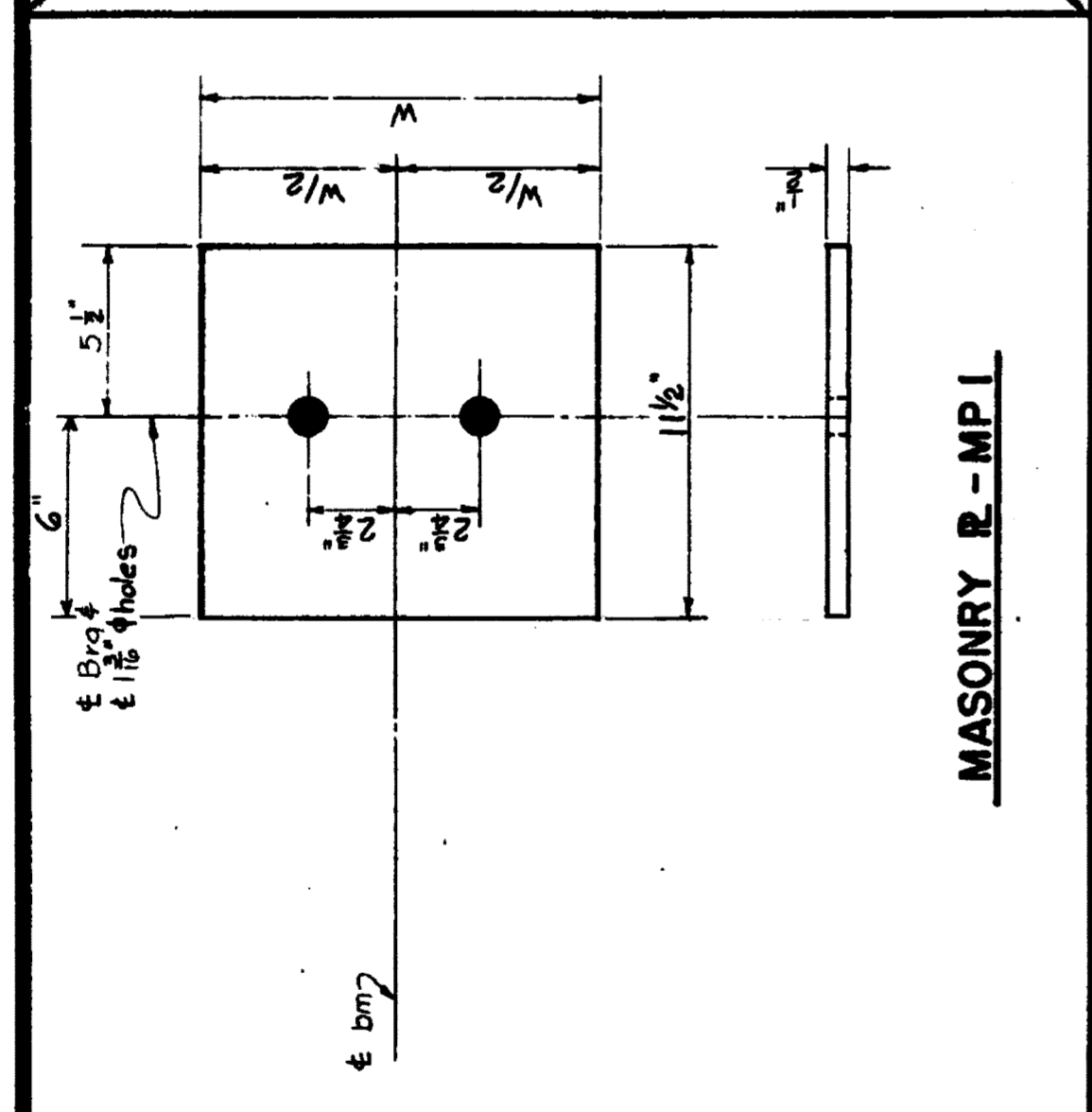
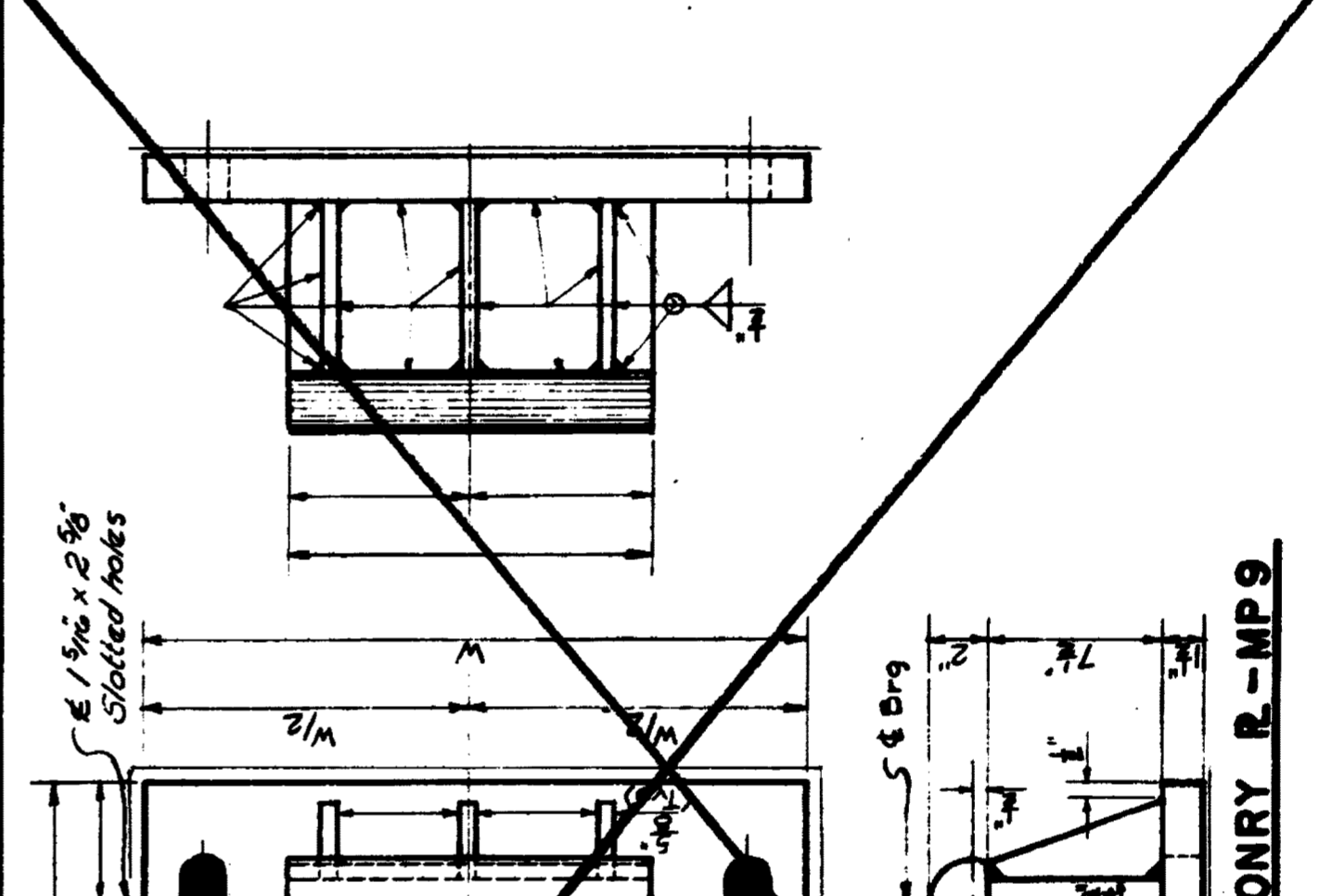
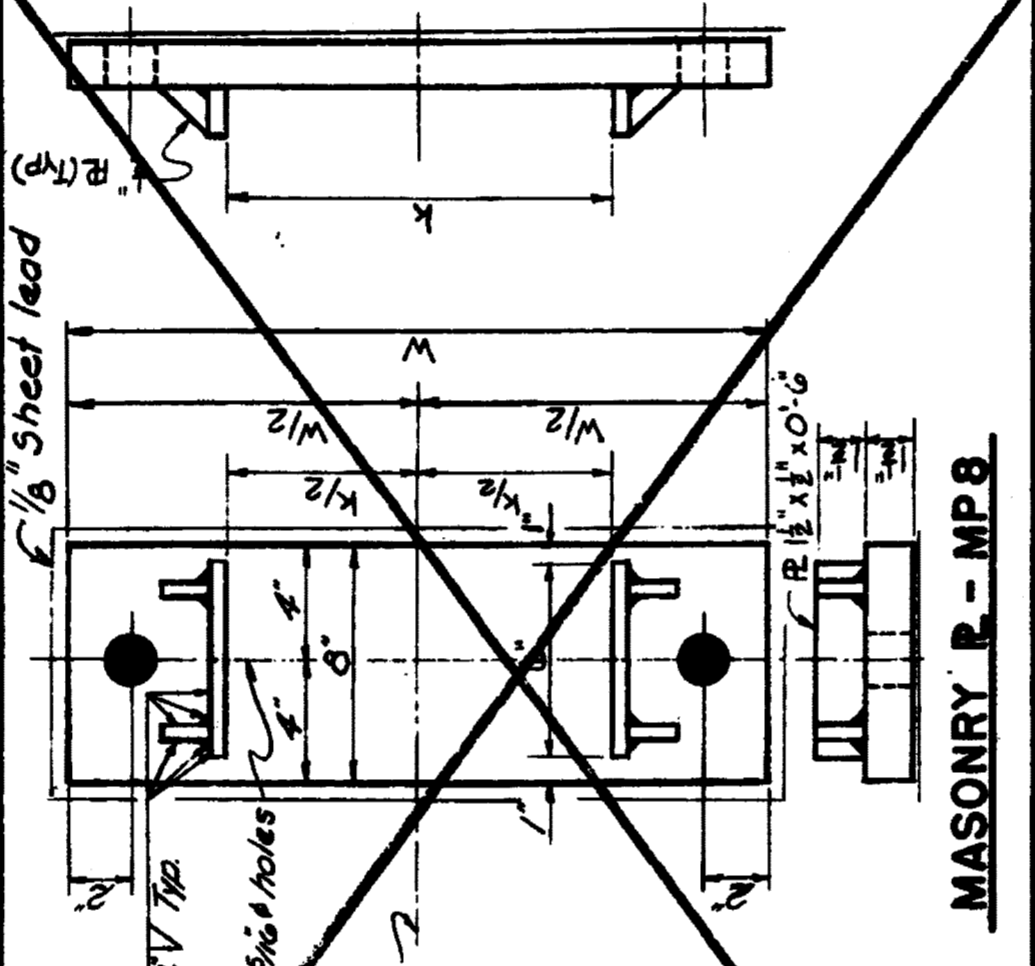
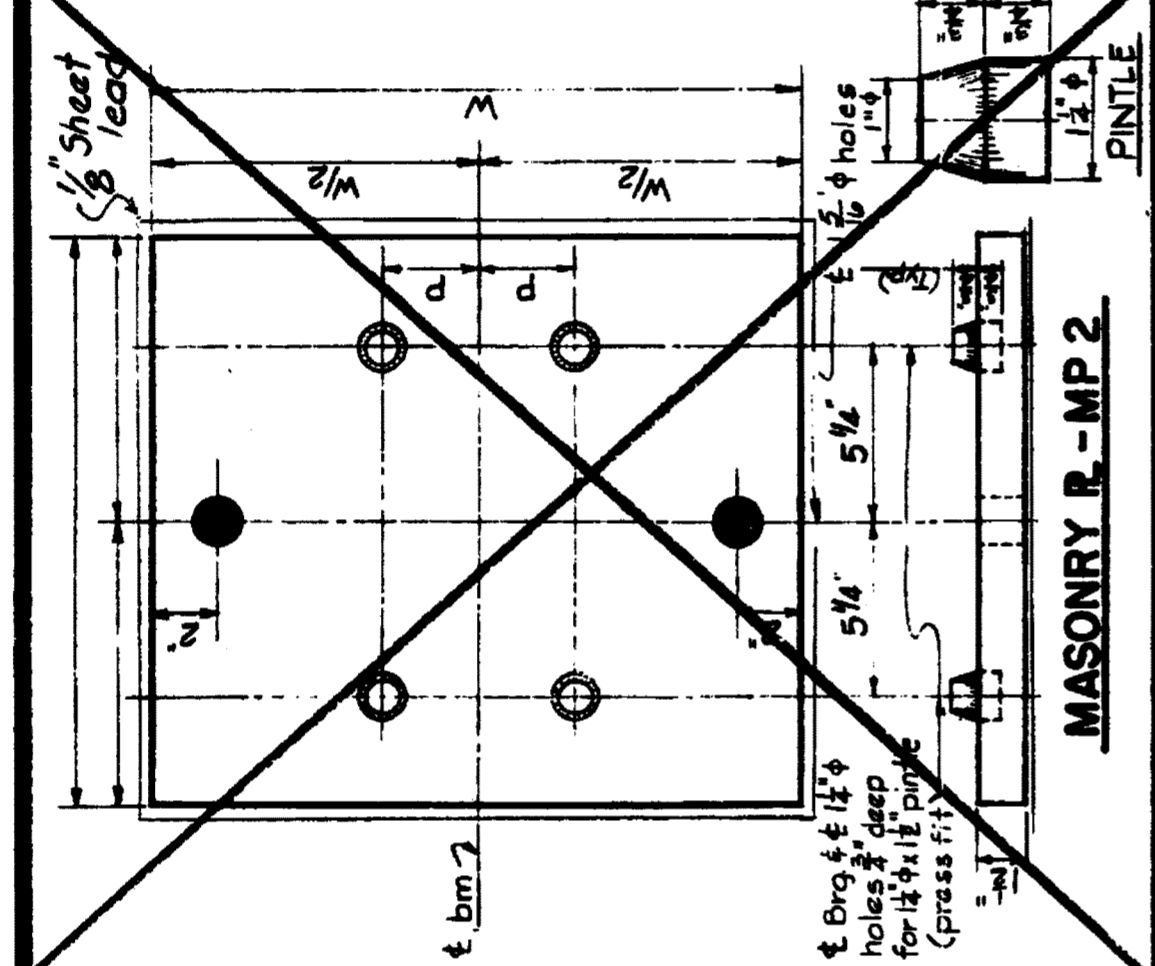
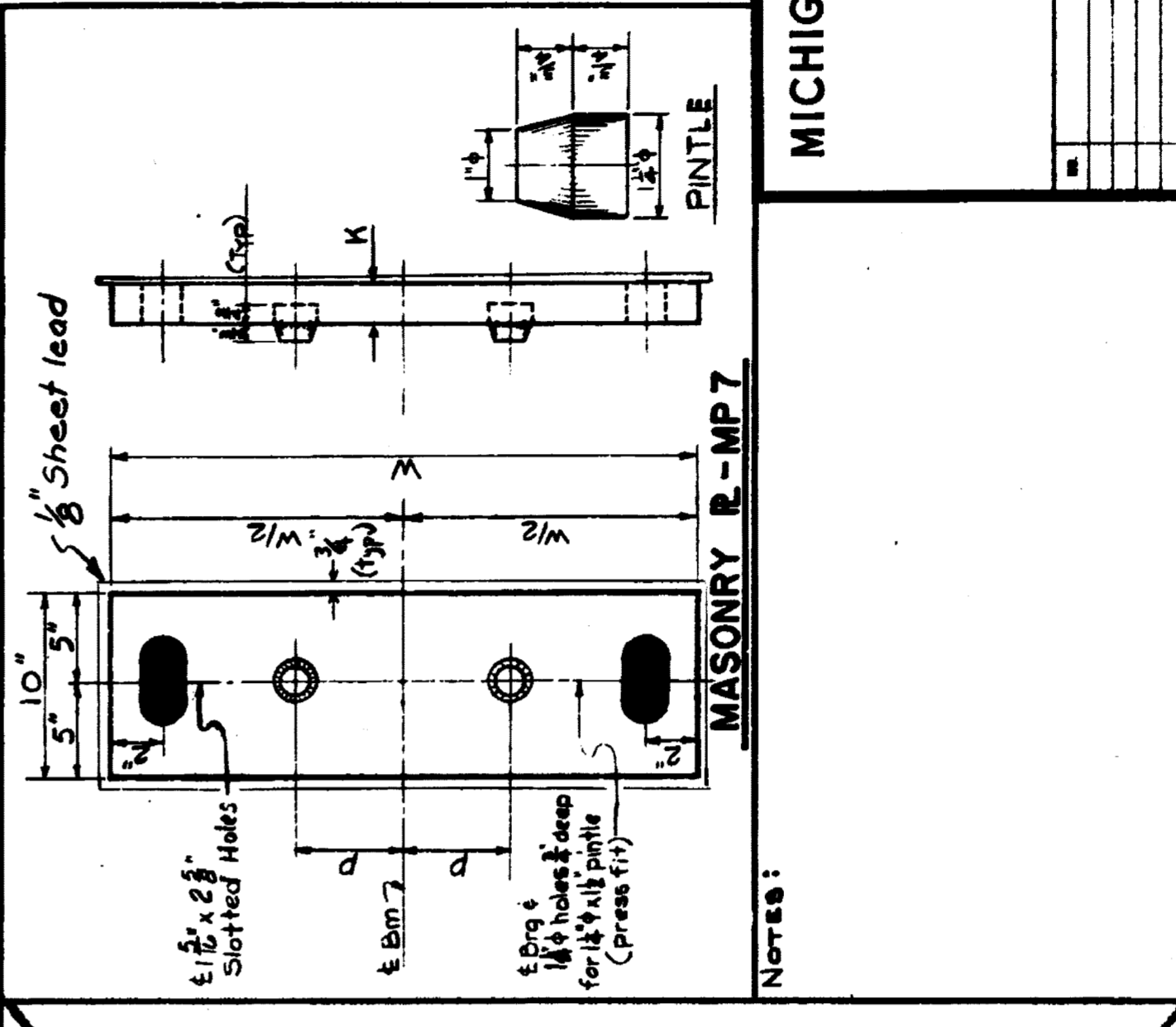
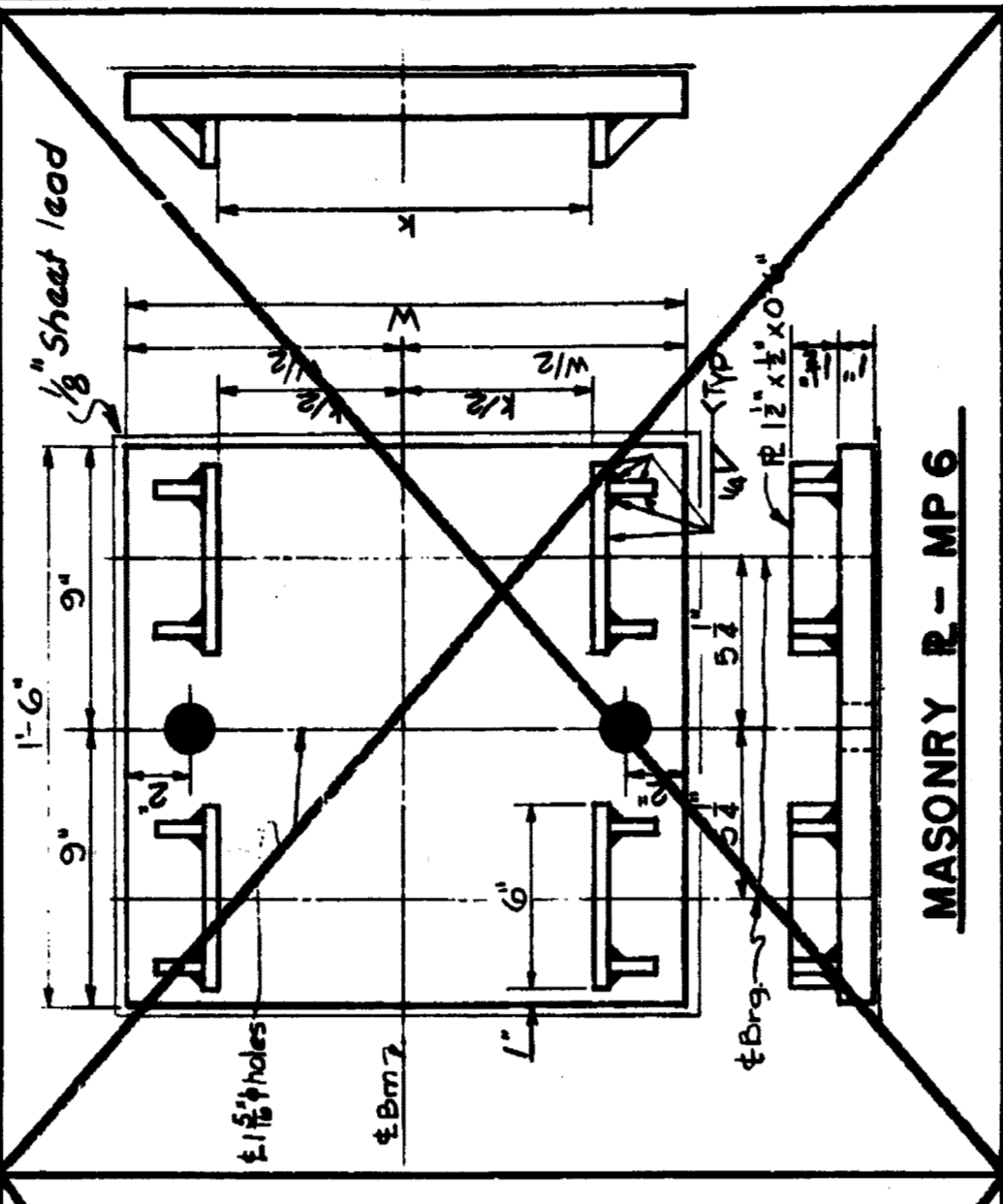
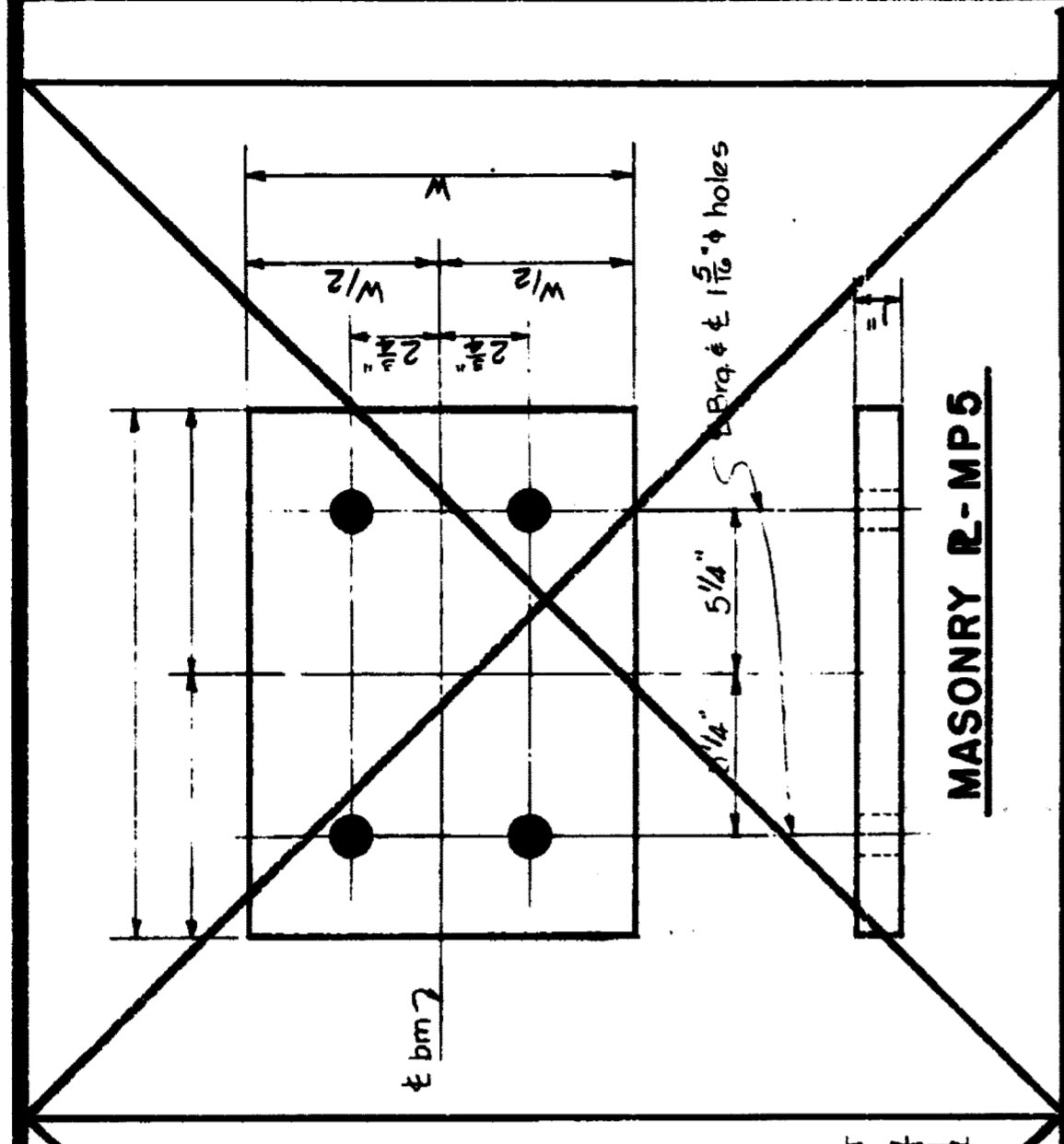
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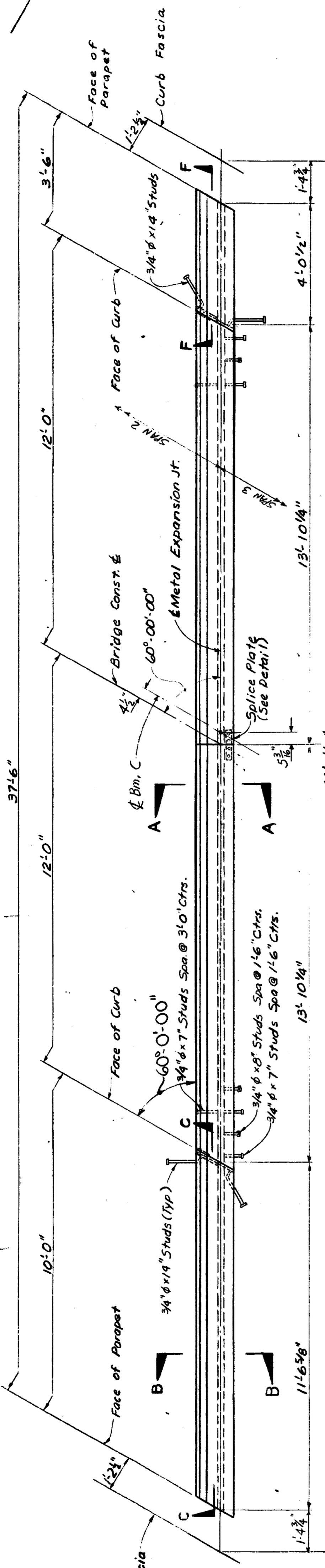
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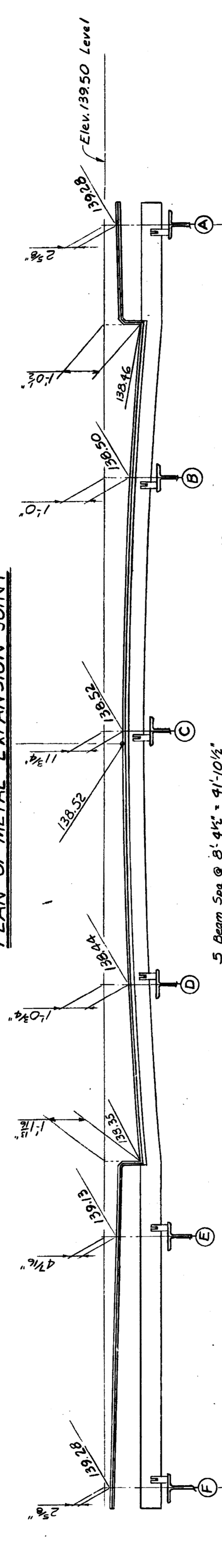
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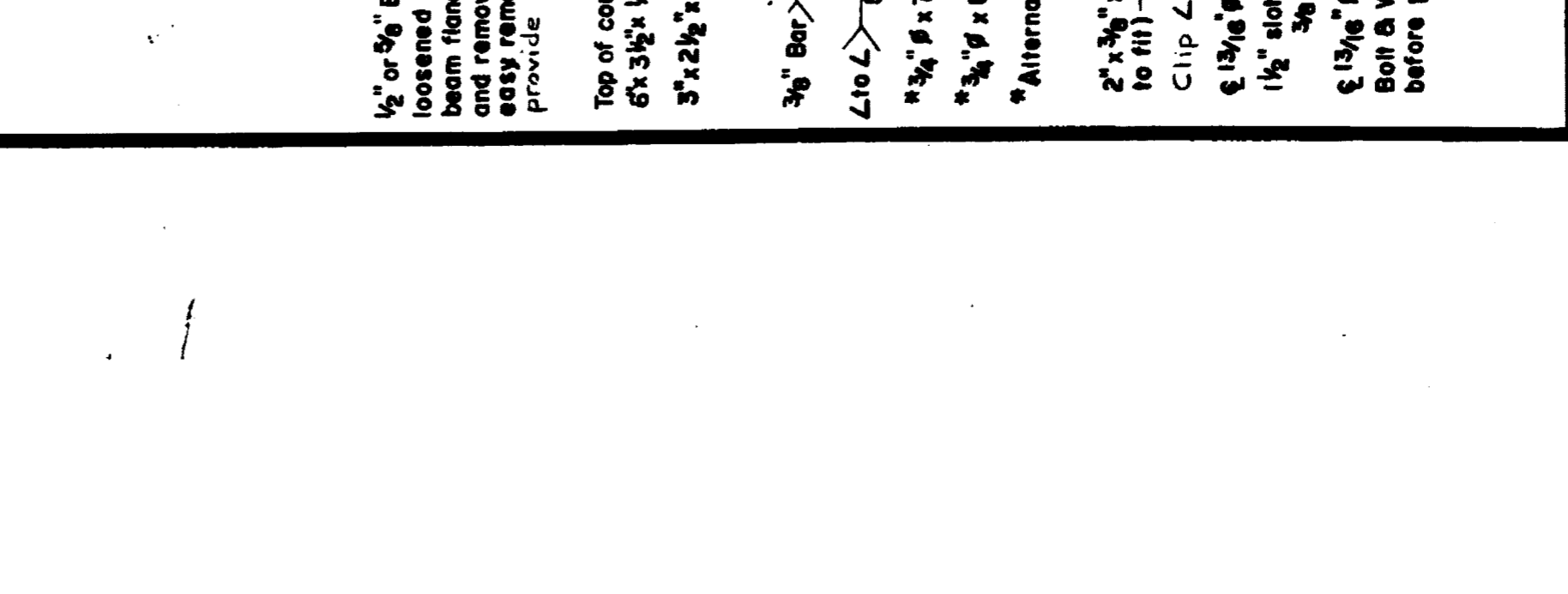
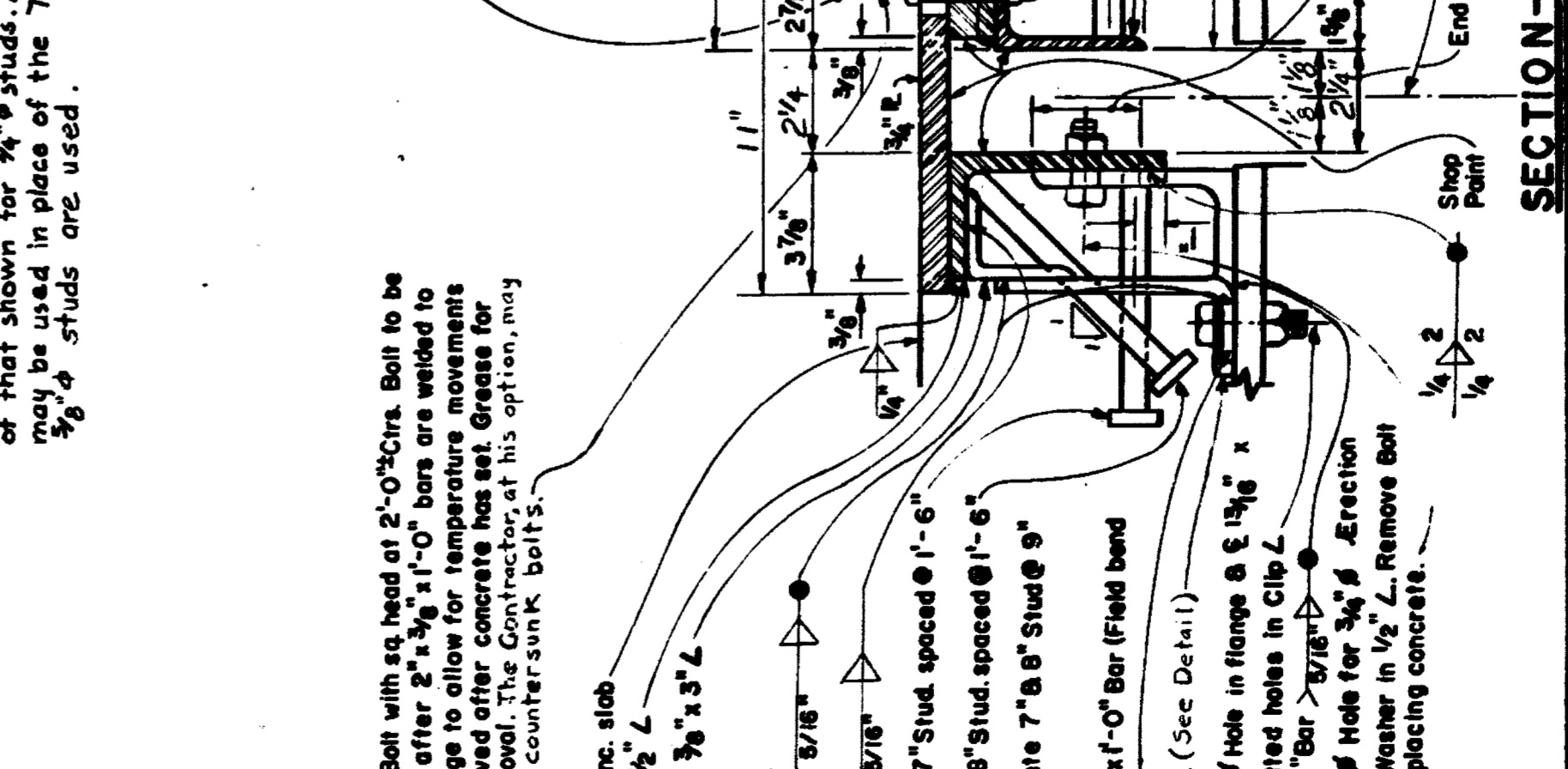
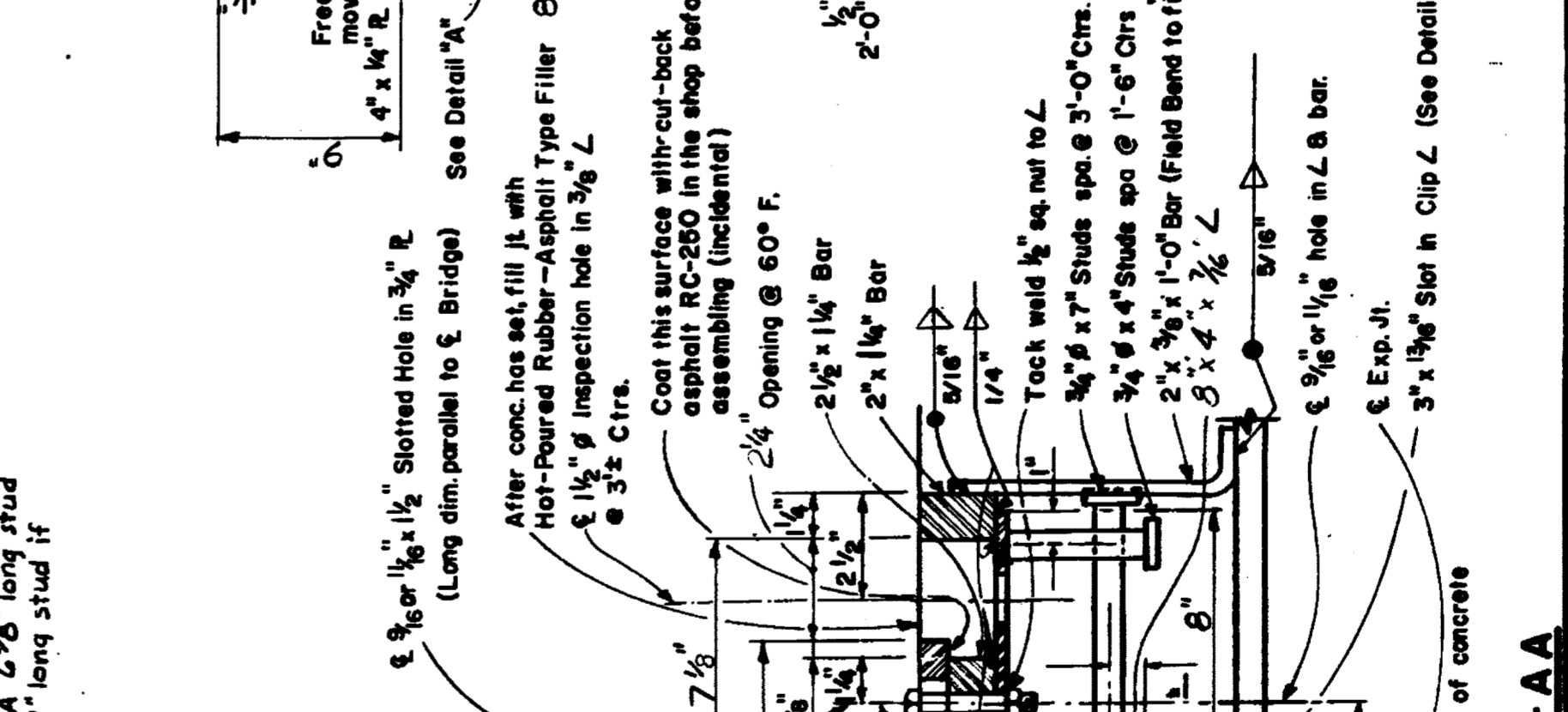
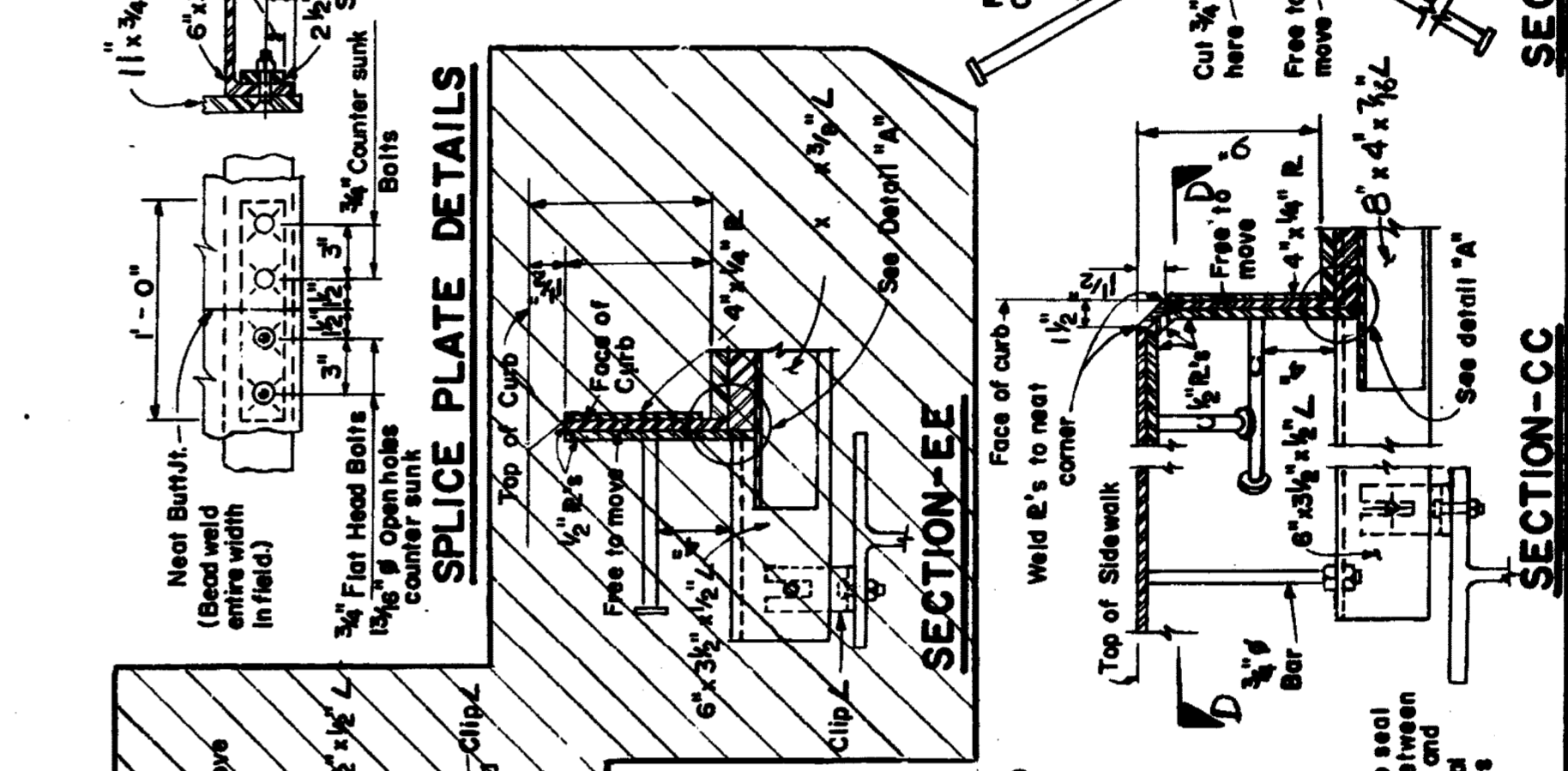


PLAN OF METAL EXPANSION JOINT



3/8" studs may be used instead of 3/4" studs. The spacing of the 3/8" studs shall be 2" from that shown for 3/4" studs. A 2" long stud may be used in place of the 7" long stud if 3/8" studs are used.

TEMPLATE

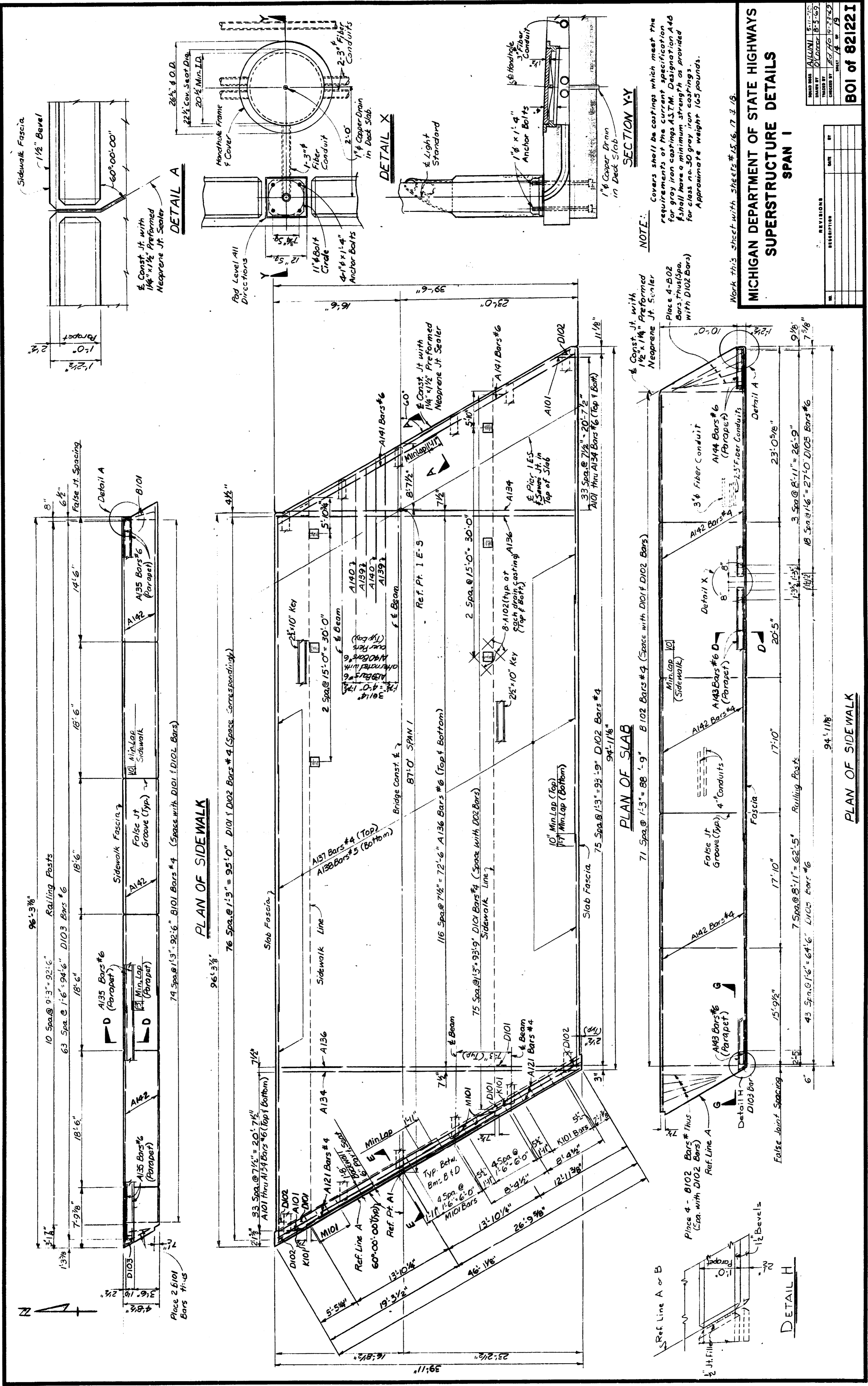


NOTES:
 The Metal Expansion Joint shall be bent in the shop to conform with the contour of the top of roadway slab.
 Hot-Poured Rubber-Asphalt Type Filler is included in the Superstructure Quantities on sheet 17.
 Weight of the Metal Expansion Joint is included in the Structural Steel weight on sheet 10.
 The Metal Expansion Joint shall not be painted in the shop, except as noted on this sheet.

MICHIGAN DEPARTMENT OF STATE HIGHWAYS
 METAL EXPANSION JOINT DETAILS

| NO. | REVISIONS | DATE | BY |
|-----|-----------|--------|-----|
| 1 | AS SHOWN | 5-1-72 | ... |
| 2 | ... | ... | ... |
| 3 | ... | ... | ... |
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| 6 | ... | ... | ... |
| 7 | ... | ... | ... |
| 8 | ... | ... | ... |

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NOTE:
Covers shall be castings which meet the requirements of the current specification for gray iron castings ASTM Designation A48. They shall have a minimum strength as provided for class no. 30 gray iron castings. Approximate weight 16.5 pounds.

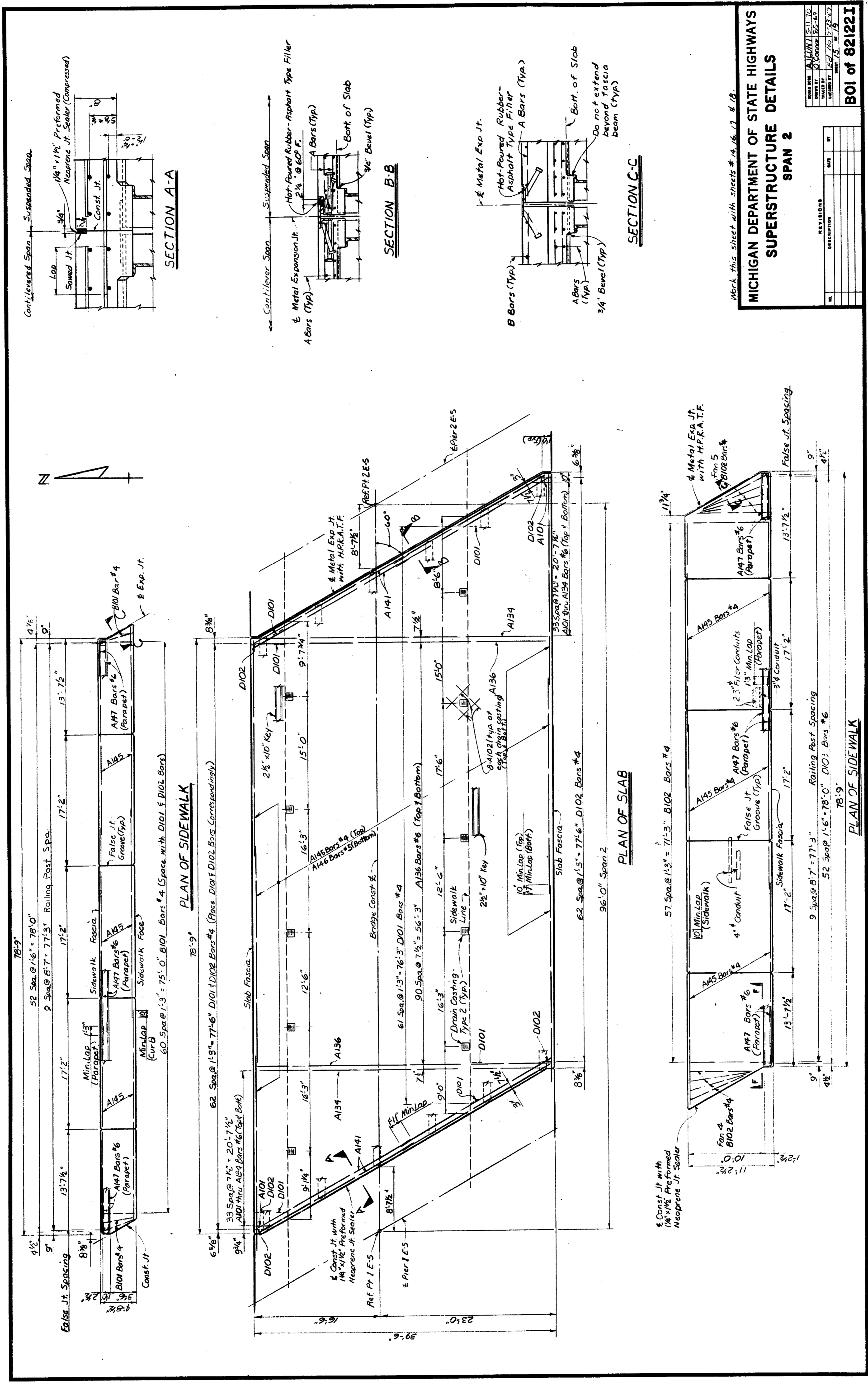
MICHIGAN DEPARTMENT OF STATE HIGHWAYS
SUPERSTRUCTURE DETAILS
SPAN I

Mark this sheet with sheets #15, 16, 17 & 18.

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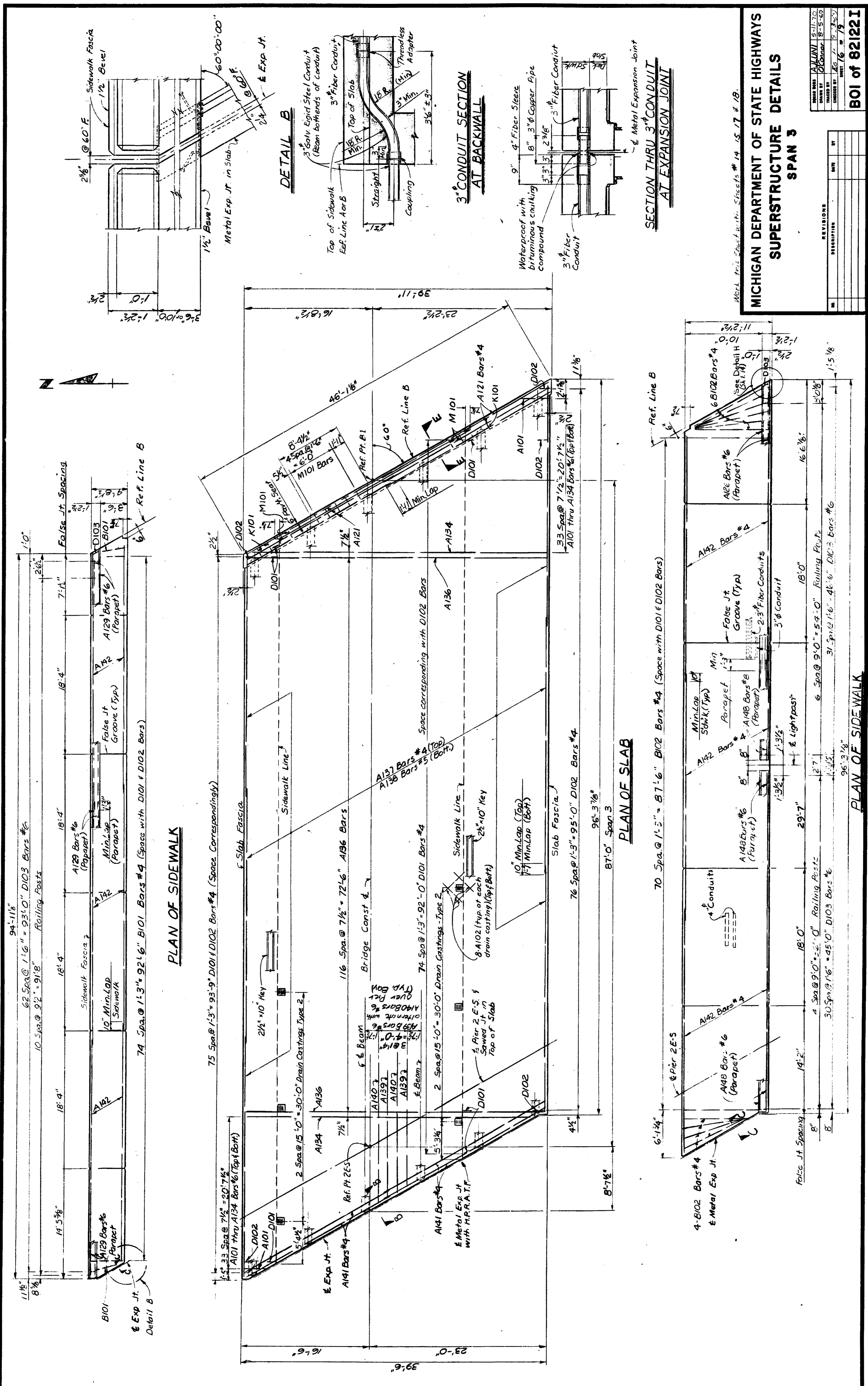
Work this sheet with sheets # 14, 16, 17 & 18.

MICHIGAN DEPARTMENT OF STATE HIGHWAYS
SUPERSTRUCTURE DETAILS
SPAN 2

| NO. | REVISIONS | DATE | BY |
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BOI of 821221

012508

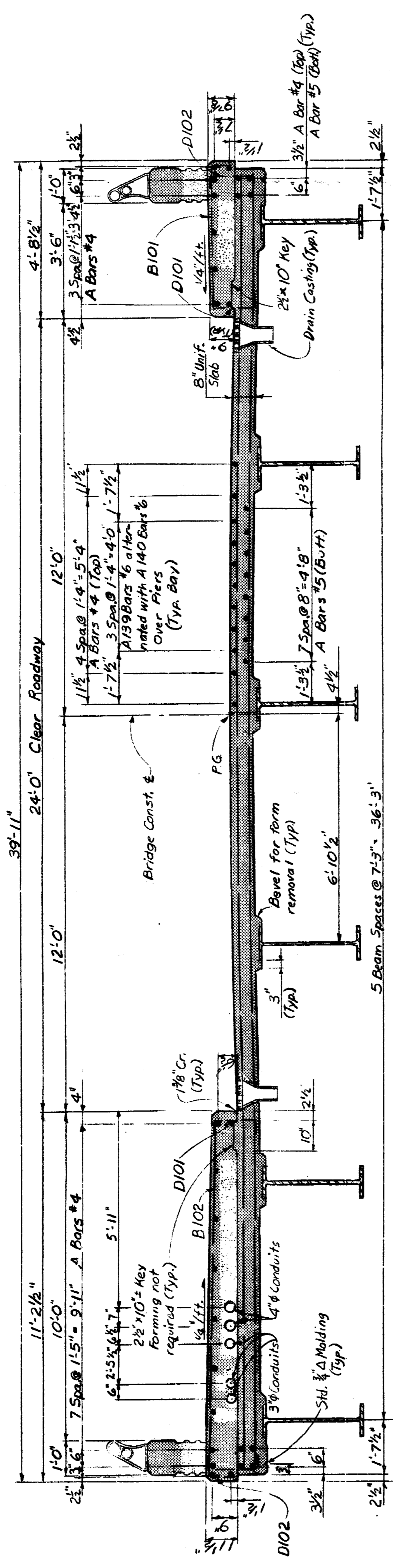


MICHIGAN DEPARTMENT OF STATE HIGHWAYS
SUPERSTRUCTURE DETAILS
SPAN 3

Mark this Sheet with Sheets # 14, 15, 17 & 18.

| NO. | REVISIONS | DATE | BY |
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DATE: 7/6/79
 DRAWN BY: J.E. ...
 CHECKED BY: ...
 PROJECT NO.: BOI of 821221
 SHEET NO.: 012308



TRANSVERSE SECTION
Looking West

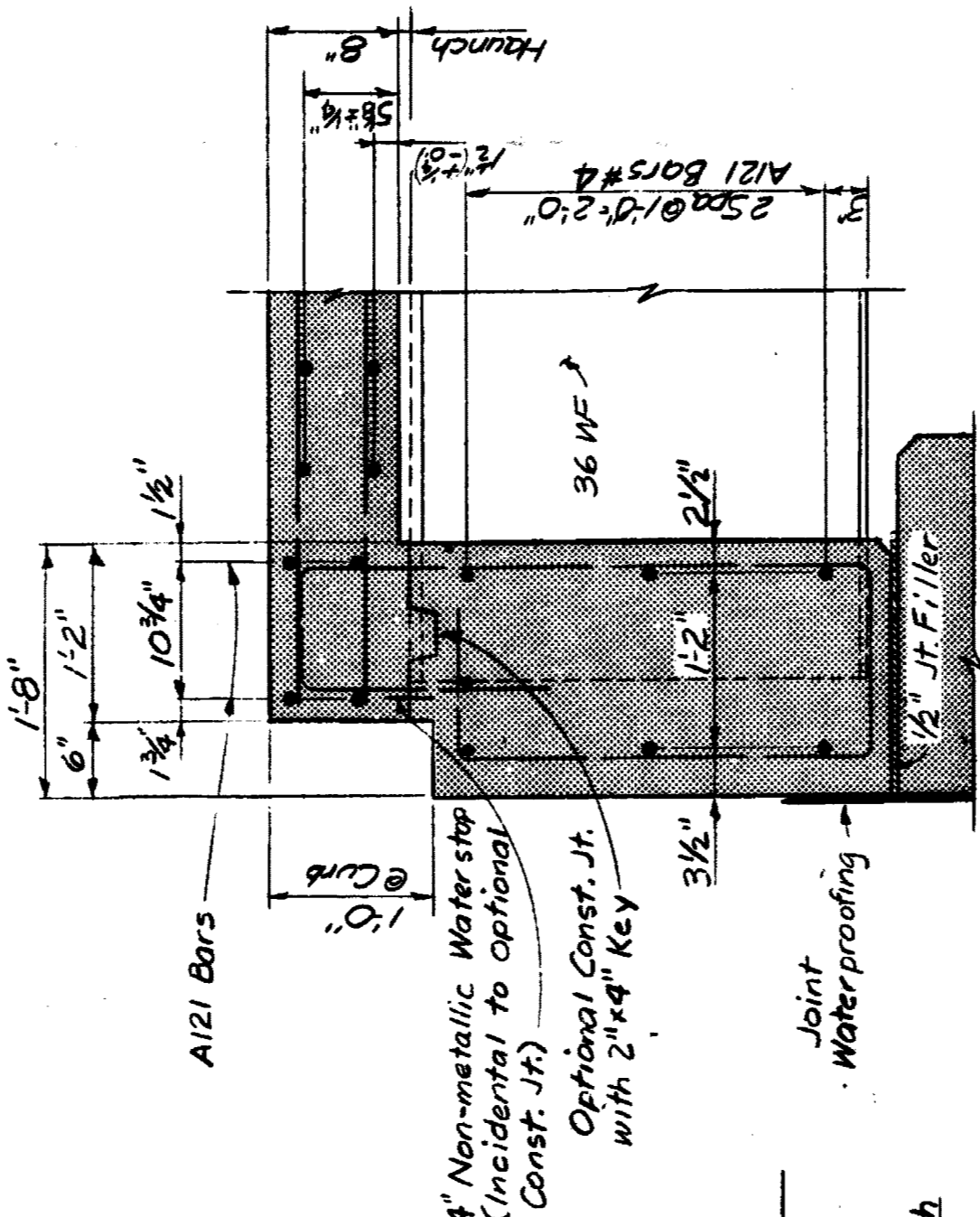
CONCRETE QUANTITIES
Grade (ALMA)

| | |
|--------------|----------------|
| FOUR | AMT. |
| A | 9728.4 |
| B | 80.5 |
| C | 97.8 |
| D | 33.6 |
| E | 21.1 |
| F | 33.6 |
| G | 13.2 |
| H | 10.9 |
| J | 13.2 |
| K | 9.4 |
| L | 9.4 |
| TOTAL | 42716.9 |

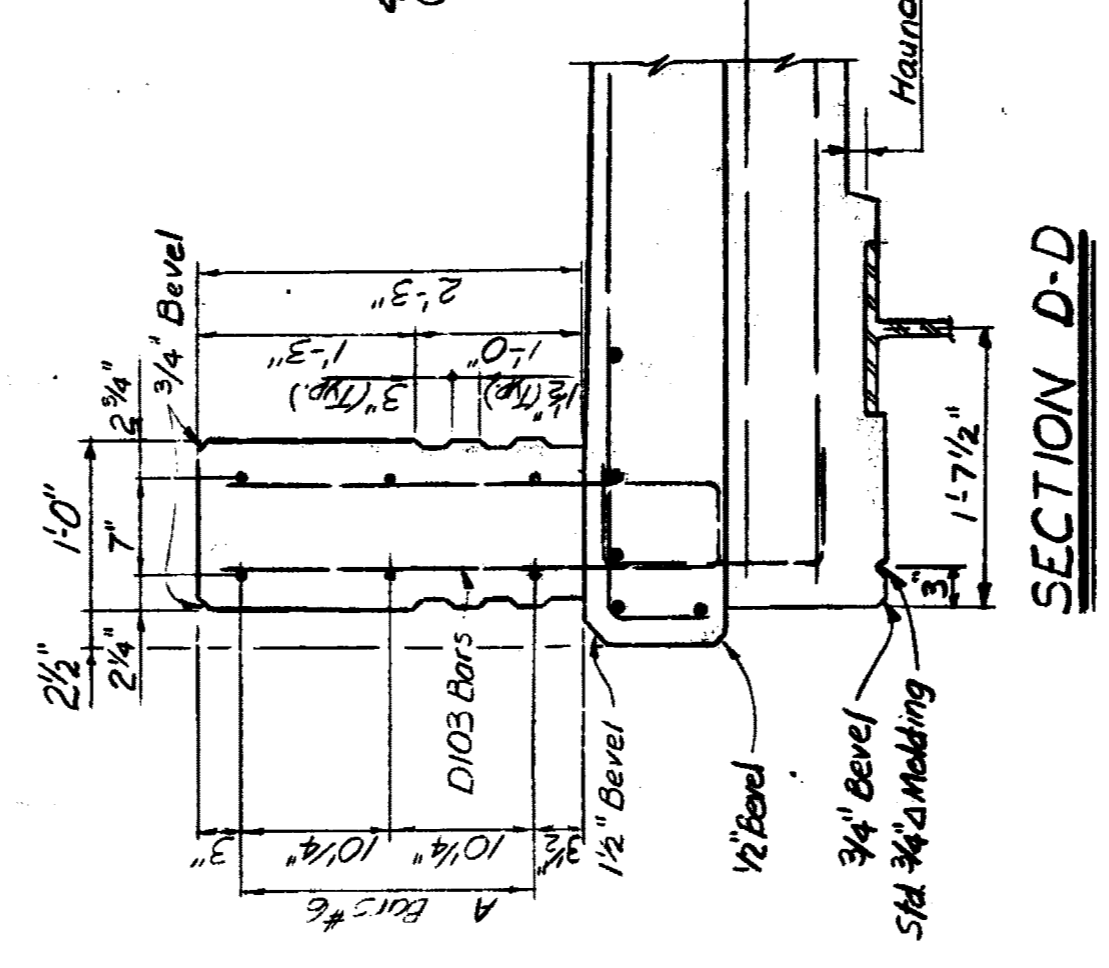
MISCELLANEOUS QUANTITIES

| | | |
|---------------------------------------|---------|-------|
| ITEM | UNIT | AMT. |
| 4" Ø Conduit | Lin.Ft. | 540 |
| 3" Ø Conduit | Lin.Ft. | 510 |
| Handhole Frame & Cover | Each | 2 |
| Light Standard Anchorage Assembly | Each | 2 |
| Protective Treatment for Bridge Decks | Sq. Ft. | 10125 |
| 1 1/4" Preformed Neoprene Jt. Sealer | Lin.Ft. | 46 |
| Hot-Fused Rubber-Asphalt Type Filler | Lin.Ft. | 46 |
| Water-Bearing Retarding Admixture | Gal. | 43 |
| Drain Casting - Type 2 | Each | 22 |
| Bridge Railing - Solid parapet Type | Lin.Ft. | 540.0 |
| 2 Tubes | | |

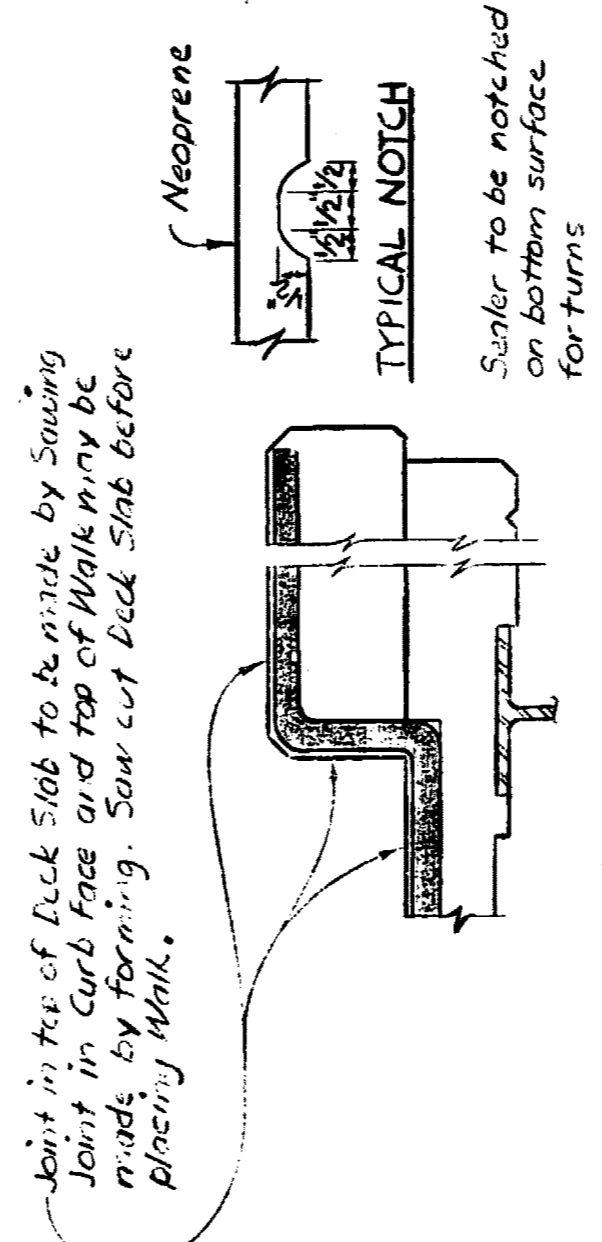
NOTES:
H.P.R.A.T.F. denotes Hot-Fused Rubber-Asphalt Type Filler.
For details of drain castings, bevels, moldings and Bridge Railing, see Standard Sheet R16.
Edge or groove denotes edging or grooving with an approved tool.
Slabwalk pours shall not be cast until slab concrete has attained a least 50% of its design strength as determined by Table 7.01-5 of the Standard Specifications.
The District Utility Engineer is to be notified one week in advance of the time of installation of the ducts in the sidewalk.
The contractor is to provide a sawed Jt. 1/2" deep by 1/8" wide (min) in the top of slab over and parallel to the centerline of piers. The joint is to be sawed before casting of sidewalk and is to be filled with H.P.R.A.T.F. (incidental) Bridge Railing is to be aluminum.
Tubular railing on concrete parapet. See Bailing Standard R16.
Protective Treatment for Bridge Decks is to be applied to all superstructure concrete surfaces between inside faces of parapets.
For name plate mounting details, see Standard Sheet R16.
For location of name plate, see sheet #3.
Anchorage for guardrail is to be provided in the permit railing. For details, see Road Plans: 5th sheet R16.
Work this sheet with sheets #4, 5, 16 & 18.



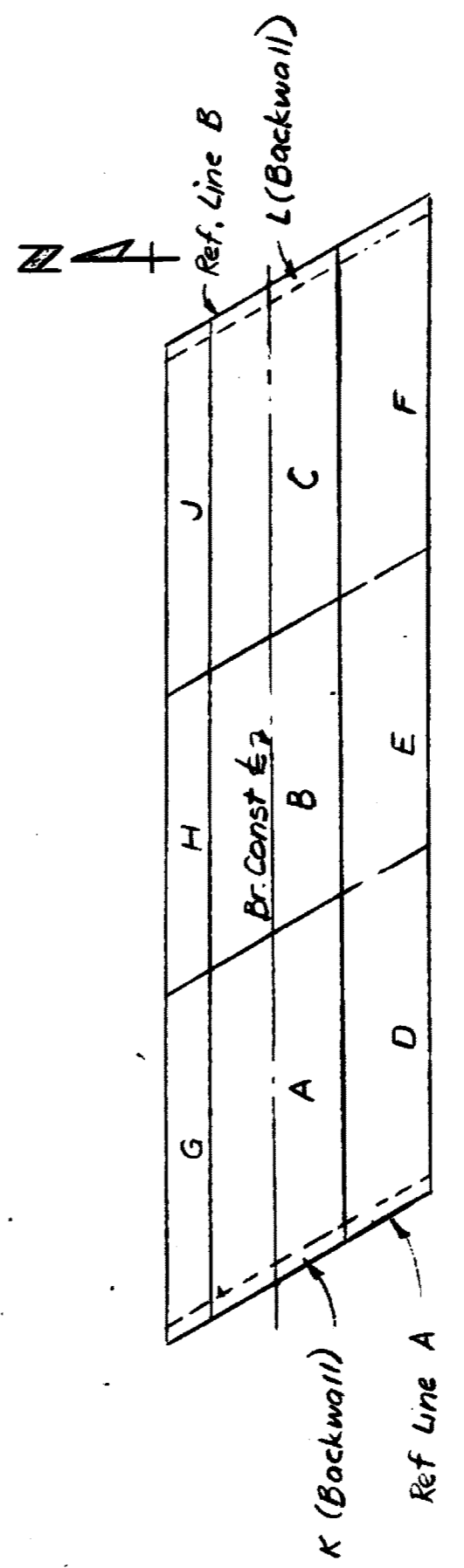
SECTION E-E



SECTION D-D

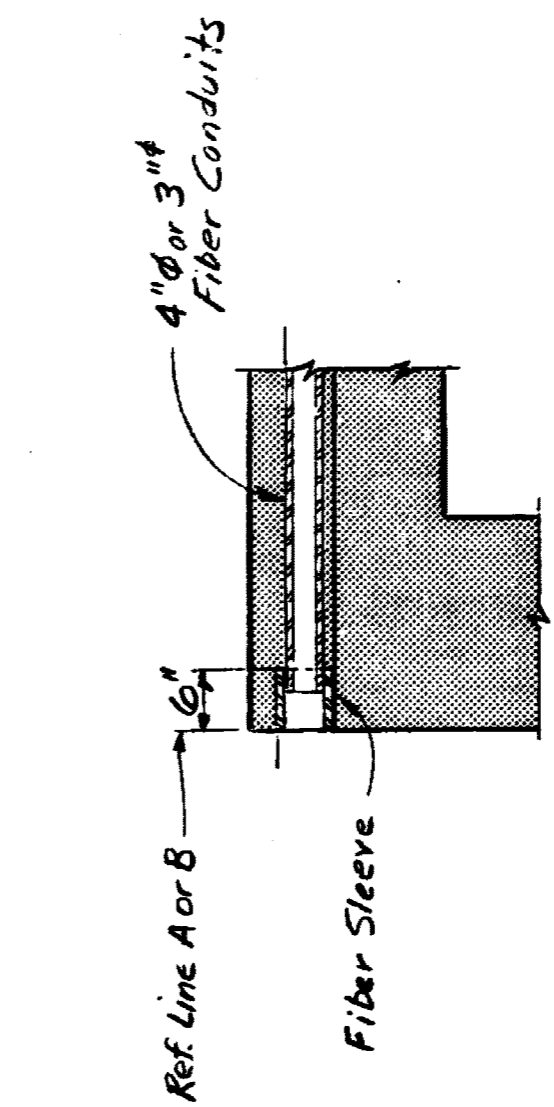


ELEVATION AT TRANSVERSE CONSTRUCTION JOINT



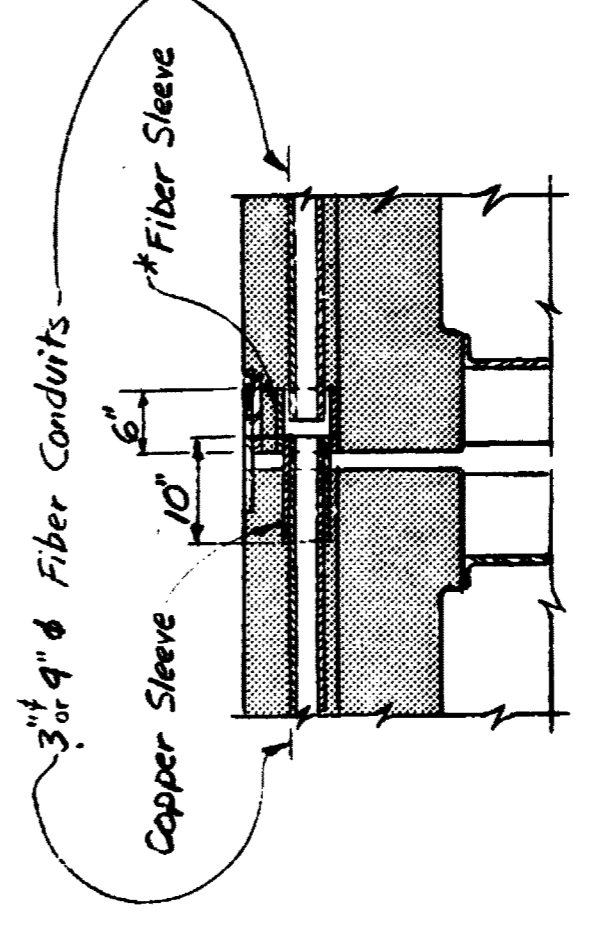
FOUR DIAGRAM

Alphabetical designation of pours is not to be construed as a pour sequence.



SECTION G-G

Section at Abutment B opposite hand



SECTION F-F

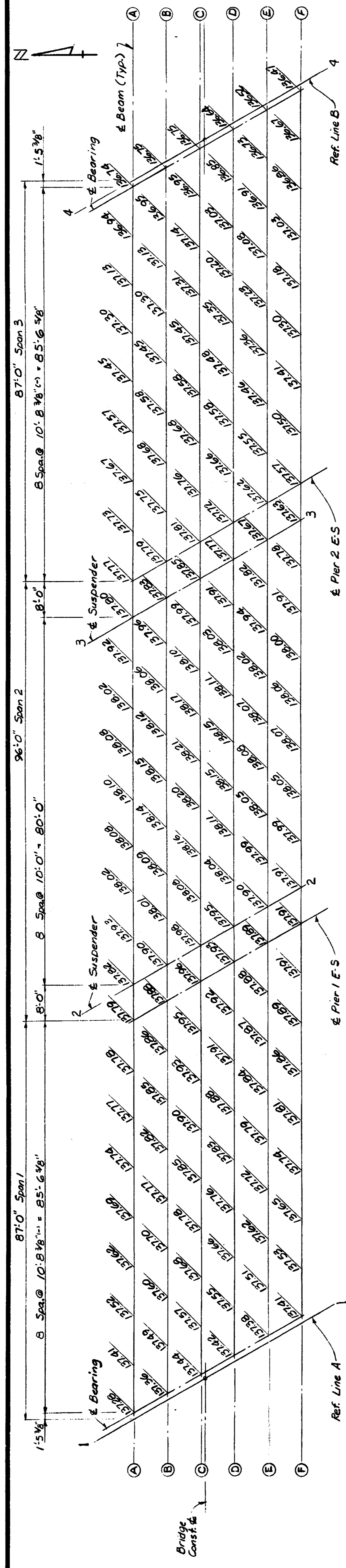
MICHIGAN DEPARTMENT OF STATE HIGHWAYS
SUPERSTRUCTURE DETAILS

BOI of 821221

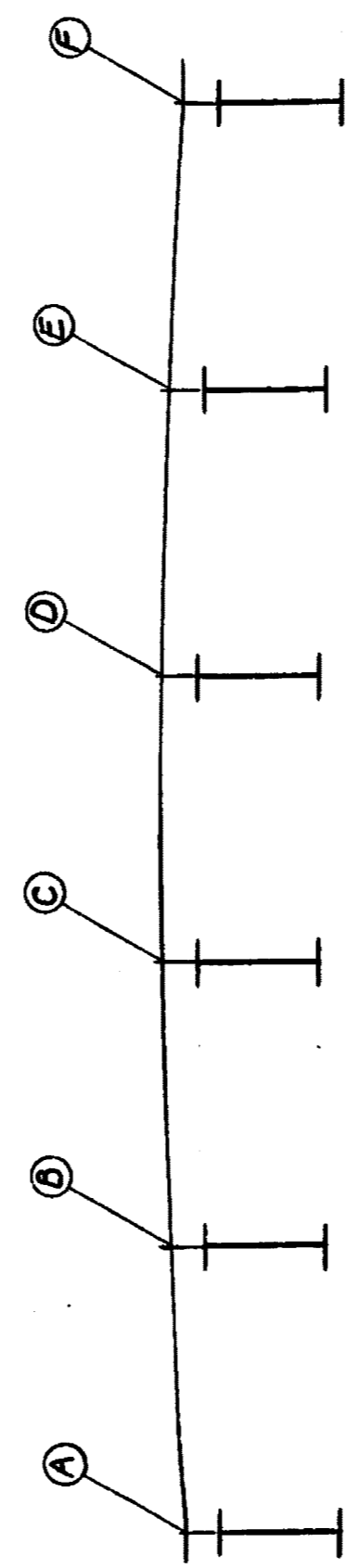
| | |
|-------------|-----------------|
| DATE | 5-11-75 |
| ISSUED BY | 060202-12152-EG |
| DESIGNED BY | 11-11-75 |
| CHECKED BY | 11-11-75 |

REVISIONS

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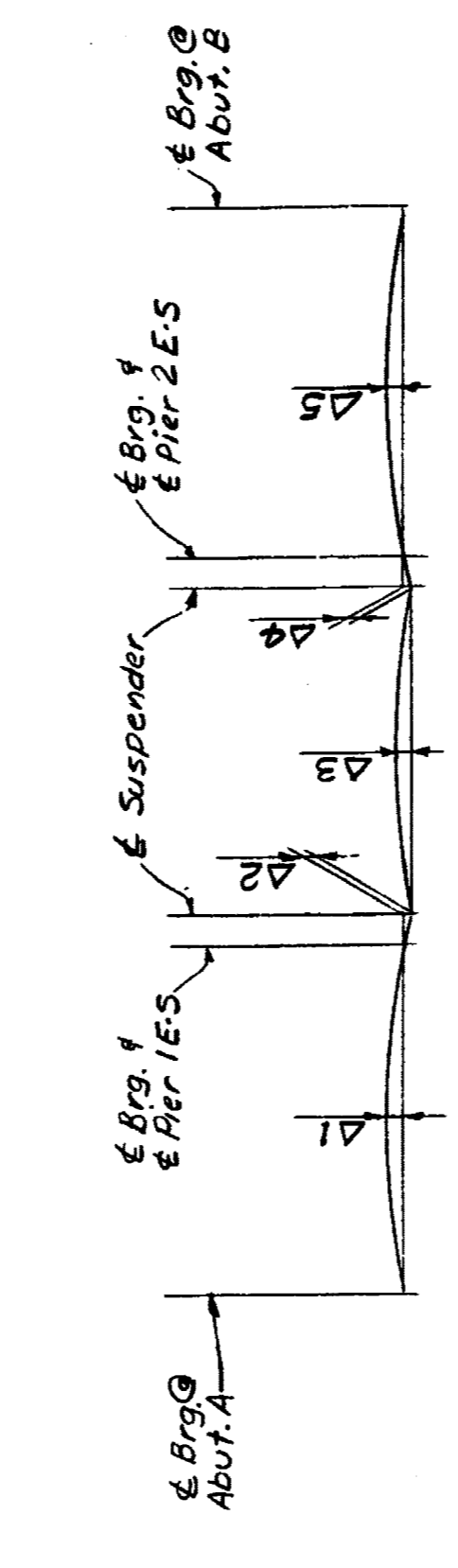


BOTTOM OF SLAB ELEVATIONS
See Section of Beam



SCREED TEMPLATE

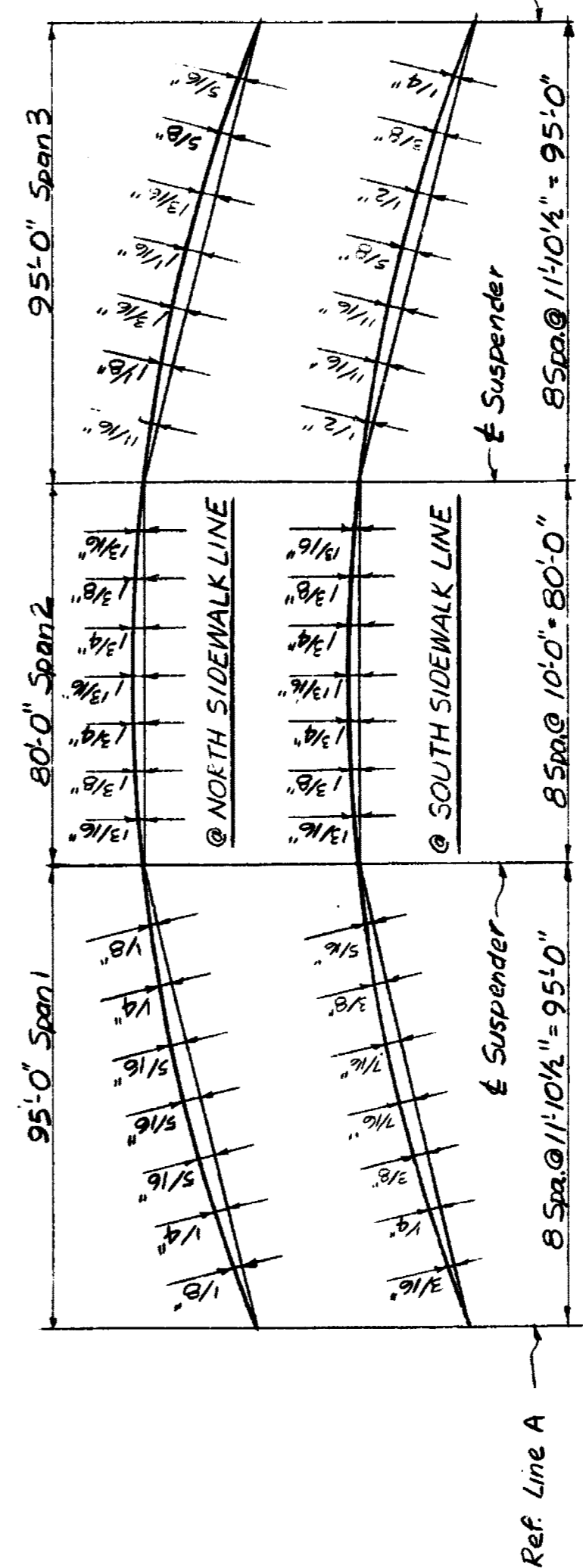
| Line | A | B | C | D | E | F |
|------|--------|--------|--------|--------|--------|--------|
| 1-1 | 137.94 | 138.02 | 138.10 | 138.08 | 138.04 | 138.06 |
| 2-2 | 138.48 | 138.56 | 138.64 | 138.61 | 138.56 | 138.58 |
| 3-3 | 138.43 | 138.50 | 138.52 | 138.44 | 138.34 | 138.30 |
| 4-4 | 137.39 | 137.39 | 137.40 | 137.39 | 137.17 | 137.12 |



CAMBER ORDINATE DIAGRAM

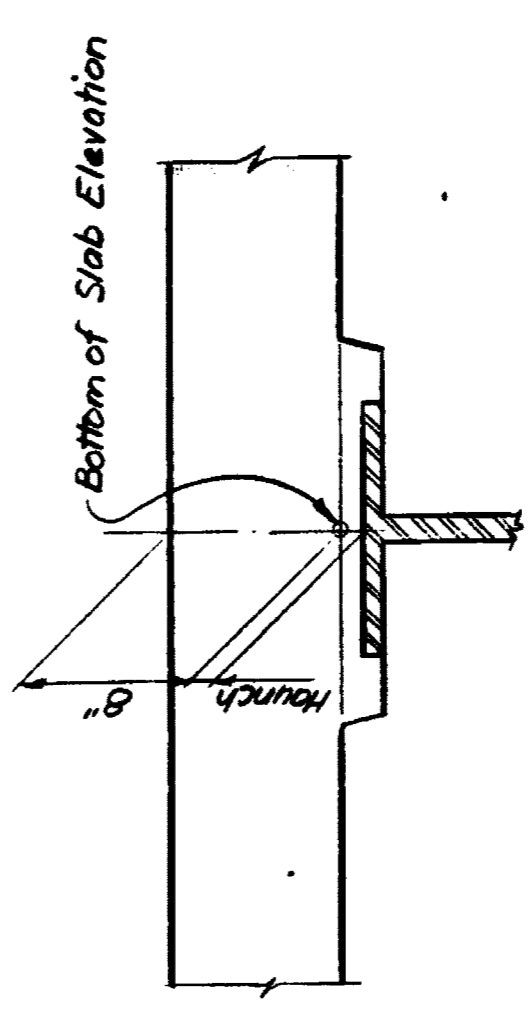
| CAMBER ORDINATES | a | b | c |
|------------------|---------|----------|--------|
| Δ1 | -2" | -1 1/16" | 0" |
| Δ2 | 1 1/16" | 3/4" | 3/8" |
| Δ3 | 3 1/2" | 3 1/4" | 1 1/2" |
| Δ4 | 1 1/16" | 3/4" | 3/8" |
| Δ5 | -2" | -1 1/16" | 0" |

NOTES:
Screeds affected by loads in other spans are to be set to the elevations shown before casting any concrete. Concrete in the suspended span is to be cast before the concrete in the anchor spans.
Screed elevations are based on the condition that no slab concrete has been cast and that framework, steel reinforcement and shear developers are in place.
Bottom of slab elevations are based on the condition that all structural steel has been erected, but no other loads applied. These elevations include allowances for deflections due to forms, steel reinforcement, shear developers in place, deck concrete and railing.



TOP OF SLAB OFFSETS

Includes Vertical Curve and allowances for deflections due to curbs, railings and future wearing surface.



TYPICAL SECTION AT EACH BEAM

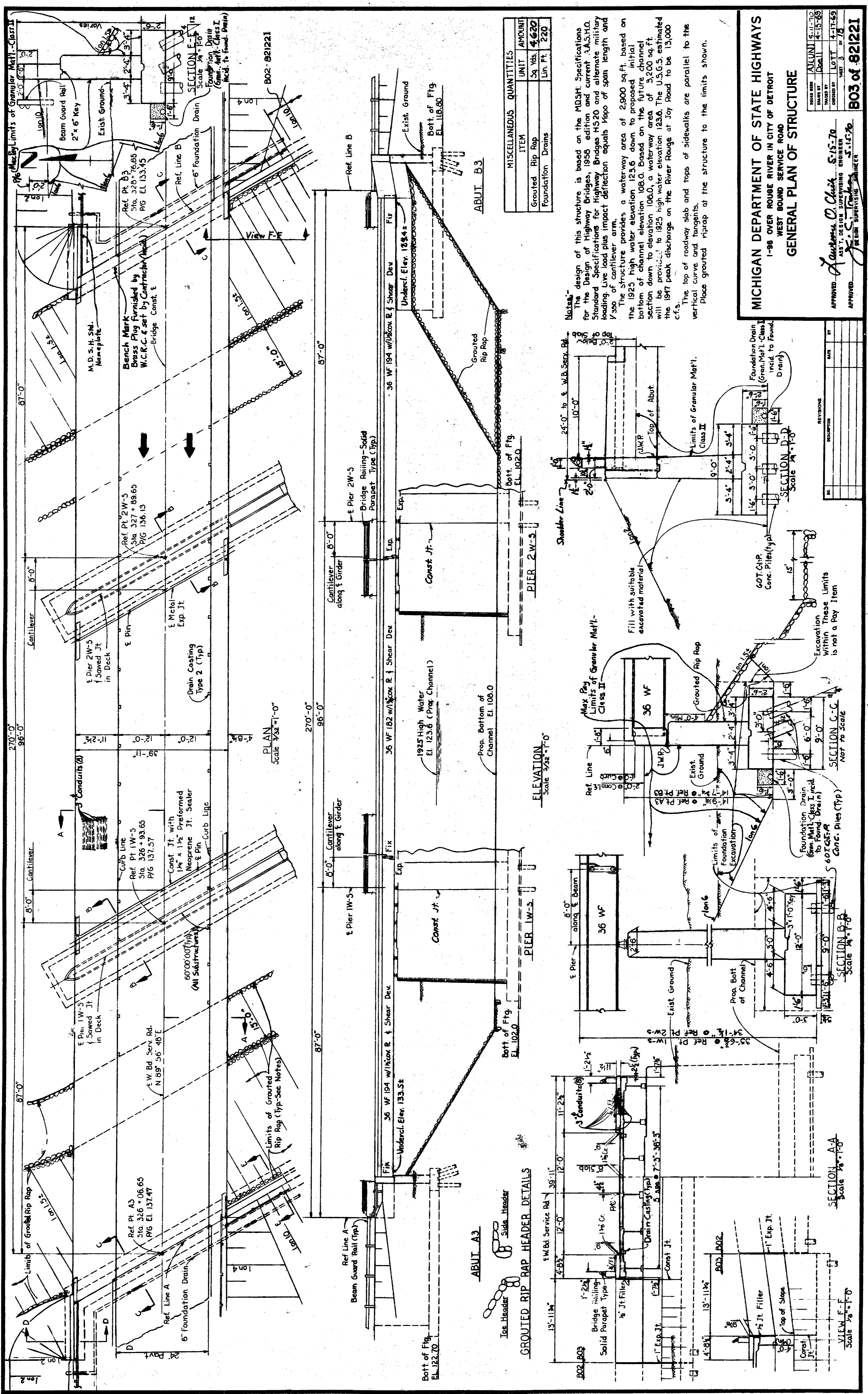
Work this sheet with sheets # 14, 15, 16 & 17.

**MICHIGAN DEPARTMENT OF STATE HIGHWAYS
SUPERSTRUCTURE DETAILS**

| NO. | REVISIONS | DATE | BY |
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|-------------|----------|-------------|---------|
| DESIGNED BY | ALUMI | DATE | 5-11-70 |
| DRAWN BY | CHANDLER | PROJECT NO. | 13-0-68 |
| CHECKED BY | ETZ | DATE | 9-27-69 |
| | | | |

BOI of 821221



MISCELLANEOUS QUANTITIES

| ITEM | UNIT | AMOUNT |
|-------------------|--------|--------|
| Grouted Rip Rap | Sq Yds | 4,620 |
| Foundation Drains | Lin Ft | 240 |

Notes:-
 The design of this structure is based on the M.D.S.T. Specifications for the Design of Highway Bridges, 1938 edition and current A.A.S.H.T.O. Standard Specifications for Highway Bridges HS20 and alternate military loading. Live load plus impact deflection equals 75% of span length and 1/350 of cantilever arm.
 The structure provides a waterway area of 2,900 sq ft, based on the 1925 high water elevation 123.6 down to proposed initial bottom of channel elevation 106.0. Based on the future channel section down to elevation 106.0, a waterway area of 3,200 sq ft will be provided to 1925 high water elevation 123.6. The U.S.G.S. estimated the 1947 peak discharge on the River Rouge at Joy Road to be 13,000 cfs.
 The top of roadway slab and tops of sidewalks are parallel to the vertical curve and tangents.
 Place grouted riprap at the structure to the limits shown.

MICHIGAN DEPARTMENT OF STATE HIGHWAYS
 1-96 OVER ROUGE RIVER IN CITY OF DETROIT
 WEST BOUND SERVICE ROAD
GENERAL PLAN OF STRUCTURE

APPROVED: *[Signature]* **5-15-76**
 ASSISTANT SUPERVISING ENGINEER

APPROVED: *[Signature]* **5-15-76**
 ASSISTANT SUPERVISING ENGINEER

DATE: 5-11-76
 DRAWN BY: D-11
 CHECKED BY: L-11
 DESIGNED BY: L-11
 SCALE: 3/32" = 1'-0"

B03 of B21221

REVISIONS

| NO. | DESCRIPTION | DATE |
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